

Part 1

Stormwater Controls Emphasizing Infiltration, Evapotranspiration, and Beneficial Uses: green roofs, cisterns, biofiltration/bioretention, porous pavement, and swales

Robert Pitt, Ph.D., P.E., BCEE, D. WRE
 Cudworth Professor of Urban Water Systems
 Department of Civil, Construction, and Environmental Engineering
 University of Alabama
 Tuscaloosa, AL, USA 35487

Watershed-Based Stormwater Controls

Multiple names for a similar goal/design process:

- Low Impact Development (LID)
- Conservation Design
- Water Sensitive Urban Design (WSUDs)
- Sustainable Urban Drainage Systems (SUDS)
- Distributed Runoff Controls (DRC)

These approaches emphasize infiltration, however, other stormwater treatment approaches will also likely be required to meet the wide range of beneficial use objectives of urban receiving waters.

Conservation Design Approach for New Development

- Better site planning to maximize resources of site
- Emphasize water conservation and water reuse on site
- Encourage infiltration of runoff at site but prevent groundwater contamination
- Treat water at critical source areas and encourage pollution prevention (no zinc coatings and copper, for example)
- Treat runoff that cannot be infiltrated at site

Modeling examples in this module will use this basic medium density residential area, based on the average of many actual neighborhoods surveyed in Wisconsin, normalized to 100 acres.

Source Area No.	Source Area	Area (acres)	H	W	P	D	S	B	Source Area Parameters
1	Roofs 1	4.500							Entered
2	Roofs 2	10.500							Entered
3	Roofs 3								
4	Roofs 4								
5	Roofs 5								
6	Paved Parking/Storage 1	0.200							Entered
7	Paved Parking/Storage 2								
8	Paved Parking/Storage 3								
9	Unpaved Pking/Storage 1								
10	Unpaved Pking/Storage 2								
11	Playground 1								
12	Playground 2								
13	Driveways 1	5.600							Entered
14	Driveways 2	1.900							Entered
15	Driveways 3								
16	Sidewalks/Walks 1	1.100							Entered
17	Sidewalks/Walks 2	1.100							Entered
18	Street Area 1	3.700							Entered
19	Street Area 2	7.600							Entered
20	Street Area 3	1.500							Entered
21	Large Landscaped Area 1	0.200							Entered
22	Large Landscaped Area 2								
23	Undeveloped Area	0.400							Entered
24	Small Landscaped Area 1	57.500							Entered
25	Small Landscaped Area 2								
26	Small Landscaped Area 3								
27	Isolated Water Body Area	0.200							Entered
28	Other Previous Area	4.000							Entered
29	Other Dis Cnctd Imp Area								
30	Other Part Cnctd Imp Area								

The file descriptions can be exported to a file under the "file" option:

```

Class examples base
SLIM Version 9.4.6
Rain file name: C:\Program Files\WinSLAMM\Rain Files\WisReg - Madison WI 1901.RAN
Particulate Solids Concentration File name: C:\PROGRAM FILES\WINSLAMM\WI_AVG01.PSC
Runoff Coefficient File name: C:\Program Files\WinSLAMM\WI_SLO6 Dec06.rsv
Particulate Residue Delivery File name: C:\PROGRAM FILES\WINSLAMM\WI_DL001.PRR
Residential Street Delivery File name: C:\Program Files\WinSLAMM\WI_Res and Other Urban Dec06.std
Institutional Street Delivery File name: C:\Program Files\WinSLAMM\WI_Com Inst Indust Dec06.std
Commercial Street Delivery File name: C:\Program Files\WinSLAMM\WI_Com Inst Indust Dec06.std
Industrial Street Delivery File name: C:\Program Files\WinSLAMM\WI_Com Inst Indust Dec06.std
Other Urban Street Delivery File name: C:\Program Files\WinSLAMM\WI_Res and Other Urban Dec06.std
Freeway Street Delivery File name: C:\Program Files\WinSLAMM\Freeway Dec06.std
Apply Street Delivery Files to Adjust the Advec Event Load Screen: First Mass Balance: False
Pollutant Relative Concentration File name: C:\PROGRAM FILES\WINSLAMM\WI_GE001.PPD
Seed for random number generator: -3
Study period starting date: 01/01/01 Study period ending date: 12/31/01
Date: 11-01-2009 Time: 17:11:32
Fraction of each type of Drainage System serving study area:
1. Grass Swales 0
2. Undeveloped roadside 0
   Curb and Gutters, 'valleys', or sealed swales in:
3. Poor condition (or very flat) 0
4. Fair condition 1
5. Good condition (or very steep) 0
Site information:
SLU/SILT-MDRNA-Medium Density Residential, No Alleys, fair Curb & Gutter

|==== Areas for each Source (acres) =====|
Residential Commercial Industrial Other
Source Area Acres Acres Acres Acres
Roofs 1 4.500 0.000 0.000 0.000 0.000
Roofs 2 10.500 0.000 0.000 0.000 0.000
Roofs 3 0.000 0.000 0.000 0.000 0.000
Roofs 4 0.000 0.000 0.000 0.000 0.000
Roofs 5 0.000 0.000 0.000 0.000 0.000
Paved Parking/Storage 1 0.200 0.000 0.000 0.000 0.000
Paved Parking/Storage 2 0.000 0.000 0.000 0.000 0.000
Paved Parking/Storage 3 0.000 0.000 0.000 0.000 0.000
Paved Parking/Storage 4 0.000 0.000 0.000 0.000 0.000

```

The following is a summary of the source area descriptions for this area:

Class examples base.dat	area (ac)				
Roofs 1	4.5 pitched	directly connected			
Roofs 2	10.5 pitched	draining to silty soils			
Paved parking/storage 1	0.2	directly connected			
Driveways 1	5.6	directly connected			
Driveways 2	1.9	draining to silty soils			
Sidewalks/walks 1	1.1	directly connected			
Sidewalks/walks 2	1.1	draining to silty soils			
Street area 1	3.7 2.0 curb mi	30.5 ft width	smooth texture	accum rate and initial load calculated by program	
Street area 2	7.6 4.0 curb mi	31.3 ft width	intermediate texture	accum rate and initial load calculated by program	
Street area 3	1.5 0.8 curb mi	30.9 ft width	rough texture	accum rate and initial load calculated by program	
Large landscaped area 1	0.2	silty soil			
Undeveloped area	0.4	silty soil			
Small landscaped area 1	57.5	silty soil			
Isolated/water body area	0.2		isolated area generating no runoff		
Other pervious area	4	silty soil			
total residential area:	100				

This is the list of data files used with these analyses (calibrated and verified by the USGS, working with the WI DNR, using much regional data):

Current File Data

SLAMM Data File Name:
 C:\Program Files\WinSLAMM\class examples base.dat

Site Descript: SLU/SILT-MDRNA-Medium Density Residential, No Alleys, fair Curb & Gutter

Edit Seed: -3

Edit Rain File: C:\Program Files\WinSLAMM\Rain Files\WisReg - Madison WI 1901.RAN

Edit Start Date: 01/01/01 Write Season Range
 Edit End Date: 12/31/01 Start of Winter (mm/DD) End of Winter (mm/DD)

Edit Pollutant Probability Distribution File: C:\PROGRAM FILES\WINSLAMM\WI_GE001.PPD

Edit Runoff Coefficient File: C:\Program Files\WinSLAMM\WI_SLO6 Dec06.rsv

Edit Particulate Solids Concentration File: C:\PROGRAM FILES\WINSLAMM\WI_AVG01.PSC

Edit Particulate Residue Delivery File: C:\PROGRAM FILES\WINSLAMM\WI_DL001.PRR

Edit Street Delivery File (Select LU)
 Residential LU Industrial LU
 Institutional LU Other Urban LU Commercial LU Freeways
 Change all Street Delivery Files to Match the Current File

Use Cost Estimation Option: Select Cost Data File

Edit Drainage System: Data Entered

Cancel Continue

The following is the basic drainage system. This is changed automatically when swales are added as a drainage control practice.

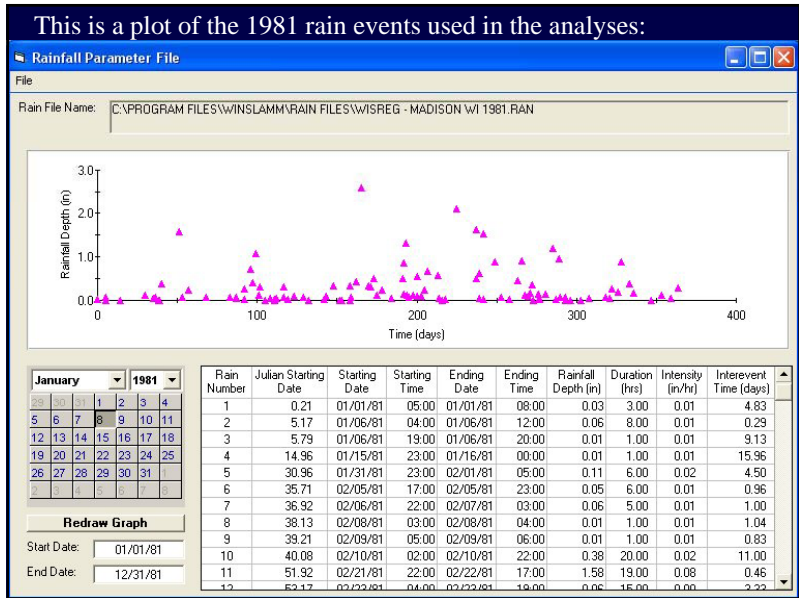
Drainage System

Enter the fraction of each type of drainage system serving the study area:

- Grass Swales:
- Undeveloped Roadside:
- Curb and Gutters, Valleys, or Sealed Swales in poor condition or very flat:
- Curb and Gutters, Valleys, or Sealed Swales in fair condition:
- Curb and Gutters, Valleys, or Sealed Swales in good condition or very steep:

Note: The grass swale drainage system fraction is calculated from the areas of the drainage system that are served by swales. These areas are entered in the Grass Swale control practice

Continue The total must equal 1. Total: 1.000
 The balance left is: 0.000



Porous Pavement and Paver Blocks

- These have long been used in Europe to reduce flows entering combined sewers.
- They are most useful in areas having little traffic (overflow seasonal parking, walkways and driveways)
- They should not be used in areas of de-icing salt applications, or in critical areas that may cause groundwater contamination



Porous paver blocks have been used in many locations to reduce runoff to combined sewer systems, thereby reducing overflow frequency and volumes.

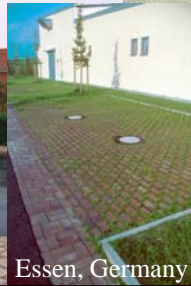
Not recommended in areas of heavy automobile use due to groundwater contamination (provide little capture of critical pollutants, plus some manufacturers recommend use of heavy salt applications instead of sand for ice control).



Madison, Wisconsin



Malmö, Sweden



Essen, Germany



Porous paver blocks and grass reinforcement material should not be used in areas having heavy parking, as the grass cannot thrive if parked on much of the time and soil compaction is still likely.

Another way to reduce runoff is to reduce the amount of paved areas. However, make sure there is sufficient parking and that the streets can handle the expected traffic load. In some cases of heavy use, cars are parked on the sidewalks, forcing pedestrians to walk in the street.



Modeling Porous Pavements

- Porous pavement (or paver blocks or turf-reinforcers) can be modeled at the source areas.
- In this example, paver blocks are used for the directly connected driveways.
- The pavers are 3 inches thick and are on another 3 inches of sand. That is in turn on 12 inches of an aggregate base for storage.
- Degradation in performance occurs with clogging. The values used are described in the help file.

Main input screen for porous pavement

Porous Pavement Control Device

Land Use: Residential
 Source Area: Driveways 1
 Total Area: 5.6 acres Porous Pavement Number 1
 Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Porous pavement area (acres): 5.60
 Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Pavement Geometry and Properties	
1- Pavement Thickness (in)	3.0
Pavement Porosity (0-1)	0.30
2- Aggregate Bedding Thickness (in)	3.0
Aggregate Bedding Porosity (0-1)	0.30
3- Aggregate Base Reservoir Thickness (in)	12.0
Aggregate Base Reservoir Porosity (0-1)	0.30

Outlet/Discharge Options

Perforated Pipe Underdrain Diameter, if used (inches)	0.00
4- Perforated Pipe Underdrain Outlet Invert Elevation (inches above Datum)	0.0
Number of Perforated Pipe Underdrains	0
Subgrade Seepage Flow (in/hr) - select below or enter	
Use Random Number Generation to Account for Uncertainty in Seepage Rate	<input type="checkbox"/>
Subgrade Seepage Rate CDV	

Select Subgrade Seepage Rate

<input type="radio"/> Sand - 8 in/hr	<input type="radio"/> Clay loam - 0.1 in/hr
<input type="radio"/> Loamy sand - 2.5 in/hr	<input type="radio"/> Silty clay loam - 0.05 in/hr
<input type="radio"/> Sandy loam - 1.0 in/hr	<input type="radio"/> Sandy clay - 0.05 in/hr
<input type="radio"/> Loam - 0.5 in/hr	<input type="radio"/> Silty clay - 0.04 in/hr
<input type="radio"/> Silt loam - 0.3 in/hr	<input type="radio"/> Clay - 0.02 in/hr
<input type="radio"/> Sandy silt loam - 0.2 in/hr	

Surface Pavement Layer Infiltration Rate Data

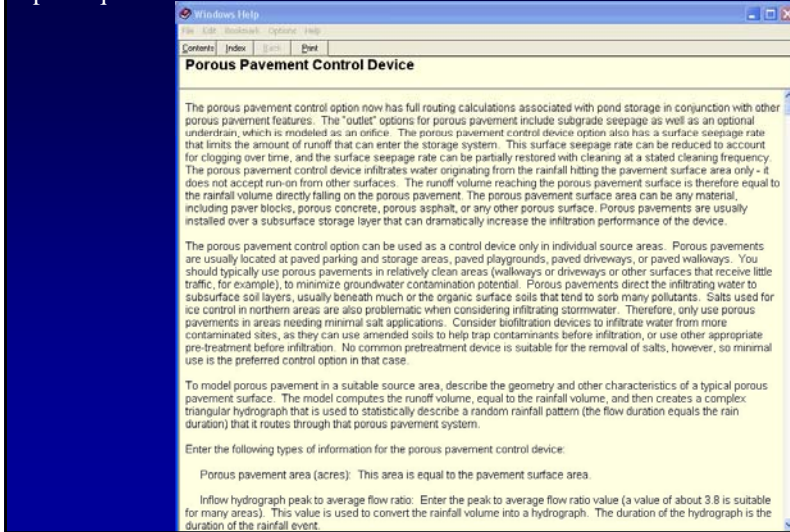
Initial Infiltration Rate (in/hr)	8.00
Percent of Infiltration Rate After 3 Years (0-100)	80.0
Percent of Infiltration Rate After 5 Years (0-100)	50.0
Percent of Original Infiltration Rate Upon Cleaning (0-100)	80.0
Time Period Until Complete Clogging Occurs (yrs)	10.0

Restorative Cleaning Frequency

Never Cleaned
 Three Times per Year
 Semi-Annually
 Annually
 Every Two Years
 Every Three Years
 Every Four Years
 Every Five Years
 Every Seven Years
 Every Ten Years

Continue Cancel Delete Control

Extensive help files (F1) are available that explain the controls and input requirements



Runoff volume summary for source areas showing complete control for the driveways that have porous paver blocks. The storage base can be reduced and re-analyzed, if desired.

Runoff Volume [cu ft]		Source Area Runoff Volume Contribution				Output Summary				
Run Total	Roofs 1	Roofs 2	Paved Parking/Storage 1	Driveways 1	Driveways 2	Sidewalk/Walks 1	Sidewalk/Walks 2	Street Area 1	Street Area 2	St Area
Minimum:	0.01	10.00	0	1.000	0	0	0	0	0	0
Maximum:	2.59	41895	19744	1722	0	2673	9679	2069	32553	66865
Average:	0.29	4419	645.4	195.0	0	116.0	979.2	67.62	3295	6765
Total:	32.10	477267	69698	16821	0	12611	105749	7304	395886	730598

Roof Runoff Control

- Runoff disconnections
- Rain gardens for roof runoff
- Green roofs to reduce flows and to provide benefits to the building
- Capture of roof runoff for beneficial uses

One of the simplest and most effective approaches for the control of stormwater is to reduce the amount of impervious areas that are directly connected to the drainage system. This can be accomplished by using less paved and roof areas (hard to do and meet design objectives), disconnect the impervious areas, or reduce the runoff from the impervious areas by infiltration, or other, methods. Reducing the runoff volume also reduces the pollutant discharges, reduces peak flows, and reduces combined sewer overflows.



Disconnected roof drain



Directly connected roof drain



Calculated Benefits of Various Roof Runoff Controls (compared to typical directly connected residential pitched roofs)

Annual roof runoff volume reductions	Birmingham, Alabama (55.5 in. annual rain)	Seattle, Wash. (33.4 in.)	Phoenix, Arizona (9.6 in.)
Flat roofs instead of pitched roofs	13%	21%	25%
Cistern for reuse of runoff for toilet flushing and irrigation (10 ft. diameter x 5 ft. high)	66	67	88
Planted green roof (but will need to irrigate during dry periods)	75	77	84
Disconnect roof drains to loam soils	84	87	91
Rain garden with amended soils (10 ft. x 6.5 ft.)	87	100	96



Green Roofing

Extensive Green Roof

- Lighter
- ≤ 6 " media depth
- Planted with sedums or native plant species
- Saturated weights from 12-50lbs/sq.ft.

Intensive Green Roof

- Heavier
- ≥ 12 " media depth
- Wider variety of plants which need more care and irrigation
- Saturated weights from 80-100lbs/sq.ft.

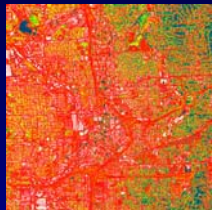
Benefits of Green Roofing



- Reduce Heat Island Effect
- Reduce Air Pollution and Greenhouse Gas Emission
- Improved human health and comfort
- Enhanced Stormwater Management and Water Quality
- Improved Quality of Life

Information courtesy of the Environmental Protection Agency – <http://www.epa.gov/heatisland/mitigation/greenroofs.htm>
<http://www.coolflatroof.com/pics/green-roof-blocks.jpg>

Urban Heat Island Effect – Atlanta, GA

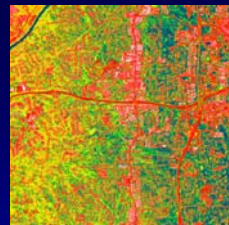


Urban Temp. - Day

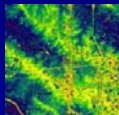


Urban Temp. - Night

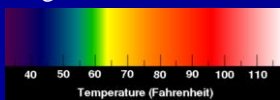
Can a green roof make the urban look like the suburban?



Suburban Temp. - Day

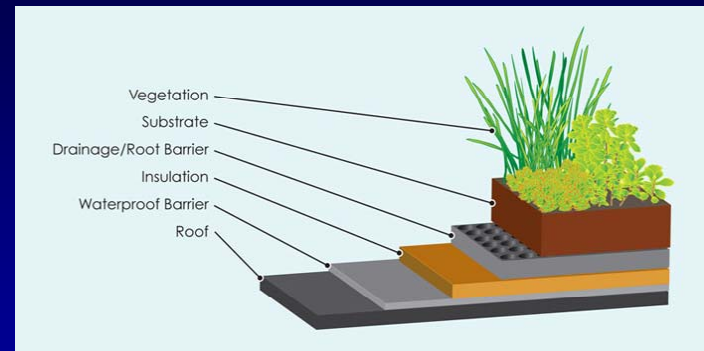


Suburban Temp. - Night



Images Courtesy of NASA

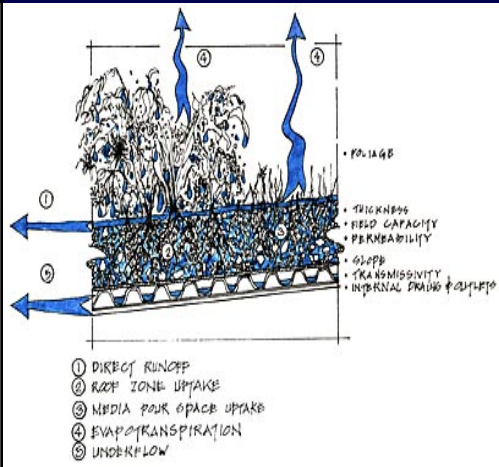
Green Roof Design



Cross-section of a typical green roof illustrating the key components

http://www.greensulate.com/green_roofs_intensive.php

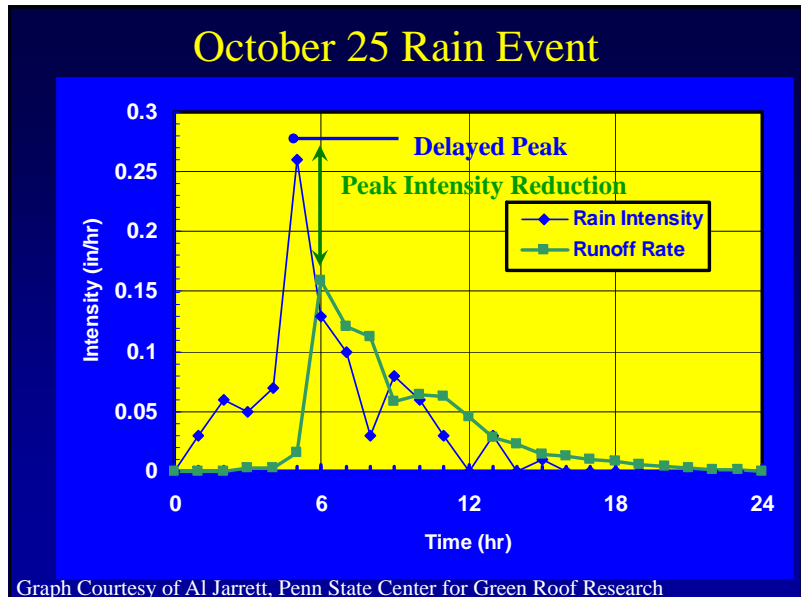
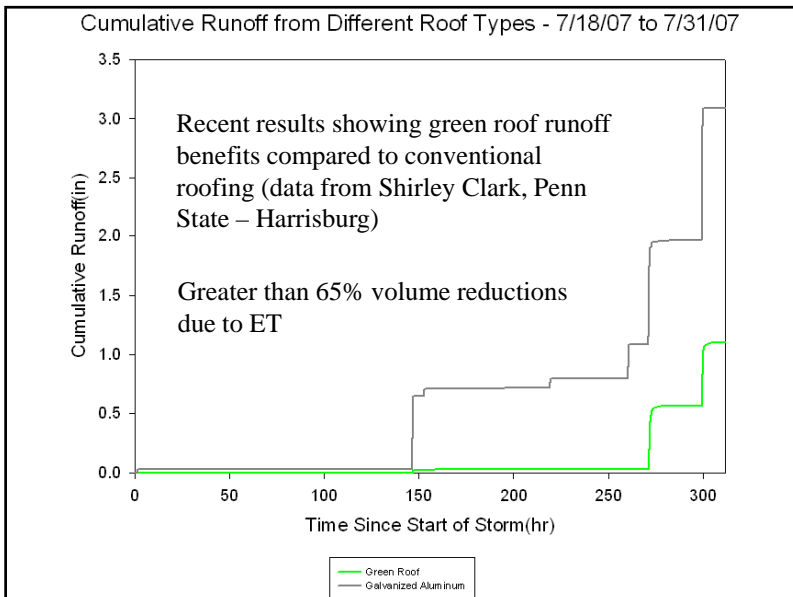
Function of a Green Roof



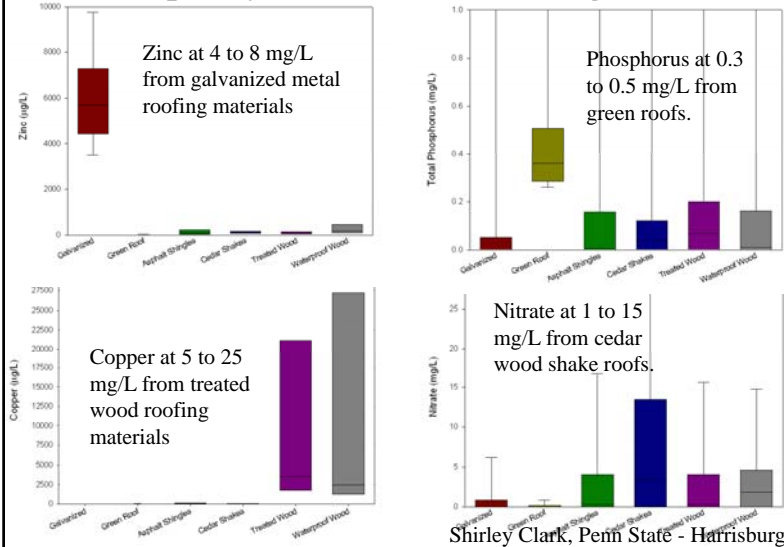
- The storage of water in the substrate
- Absorbing water in the root zone
- Capturing and holding precipitation in the plant foliage where it is returned to the atmosphere through transpiration and evaporation
- Slowing the velocity of direct runoff as it infiltrates through layers of vegetated cover

http://www.lid-stormwater.net/greenroofs_benefits.htm

Central Alabama	Average daily ET ₀ reference conditions (inches/day) (irrigated alfalfa)	Evapotranspiration (ET) is the major rain abstraction mechanism available for green roofs, besides some detention storage and evaporation.		
January	0.035			
February	0.048			
March	0.072			
April	0.102			
May	0.156			
June	0.192			
July	0.186			
August	0.164			
September	0.141			
October	0.096			
November	0.055			
December	0.036			
		Plant	Crop Coefficient Factor (Kc)	Root Depth (ft)
		Cool Season Grass (turfgrass)	0.80	1
		Common Trees	0.70	3
		Annuals	0.65	1
		Common Shrubs	0.50	2
		Warm Season Grass	0.55	1
		Prairie Plants (deep rooted)	0.50	6



Runoff quality from different roofing materials



Modeling Green Roofs

- Green roofs are modeled in WinSLAMM by using the biofiltration/bioretenion source controls.
- The device area is the area of the roofs (or less if only a portion of the roofs are to be planted).
- The broad crested weir outlet is needed to provide an overflow above a few inches of surface storage on top of the growing media.
- Evapotranspiration is the only rainfall abstraction and monthly average ET values are entered, along with “crop” factors and root depths. According to recent USGS research, the crop factors for small urban area plantings are greater than values usually published in the agricultural research.
- There is obviously no “natural soil” infiltration.

Green roof input using the biofiltration option (one unit per house).

Biofiltration Control Device

Land Use: Residential
Source Area: Roofs 1
Total Area: 4.5 acres
Biofilter Number 1

Device Properties

Top Area (sf)	1634
Bottom Area (sf)	1634
Total Depth (ft)	1.00
Typical Width (ft) (Cost est. only)	80.00
Native Soil Infiltration Rate (in/hr)	0.005
Native Soil Infiltration Rate CDV	N/A
Infil. Rate Fraction-Bottom (0-1)	1.00
Infil. Rate Fraction-Sides (0-1)	1.00
Rock Filled Depth (ft)	0.00
Rock Fill Porosity (0-1)	0.00
Engineered Soil Type	Compost Sand
Engineered Soil Infiltration Rate (in/hr)	2.10
Engineered Soil Depth (ft)	0.50
Engineered Soil Porosity (0-1)	0.35
Inflow Hydrograph Peak to Average Flow Ratio	2.00
Number of Devices in Source Area or Land Use	120

Outlet/Discharge Options

1. Sharp Crested Weir
 2. Broad Crested Weir
 3. Vertical Stand Pipe
 4. Evaporation
 5. Rain Barrel/Cistern
 6. Underdrain Outlet
 7. Evapotranspiration

Selected Outlets

1 - Broad Crested Weir
 7 - Evapotranspiration

Biointerface Schematic

Source Areas from Land Use that Contribute Runoff to Biofiltration Control Device(s)

Rooftop 1
 Rooftop 2
 Rooftop 3
 Rooftop 4
 Rooftop 5
 Paved Parking/Storage 1
 Paved Parking/Storage 2
 Unpaved Parking/Storage 1
 Unpaved Parking/Storage 2
 Playground 1
 Playground 2
 Driveway 1
 Driveway 2
 Sidewalk/Walk 1
 Sidewalk/Walk 2
 Street Area 1
 Street Area 2
 Street Area 3
 Paved Land and Shoulder 1
 Paved Land and Shoulder 2
 Paved Land and Shoulder 3
 Paved Land and Shoulder 4
 Paved Land and Shoulder 5
 Large Landscaped Area 1
 Undeveloped Area
 Small Landscaped Area 1
 Small Landscaped Area 2
 Small Landscaped Area 3
 Other Previous Areas
 Other Directly Contol Imp
 Other Partially Contol Imp
 Large Tall Area
 Undeveloped Areas
 Other Previous Areas
 Other Directly Contol Imp
 Other Partially Contol Imp

Change Geometry

Copy Biofilter Data
Paste Biofilter Data

Select Native Soil Infiltration Rate

Sand - 8 in/hr
 Loamy sand - 2.5 in/hr
 Loam - 0.5 in/hr
 Silty loam - 0.3 in/hr
 Sandy silt loam - 0.2 in/hr
 Clay loam - 0.1 in/hr
 Silty clay loam - 0.05 in/hr
 Silty clay - 0.05 in/hr
 Clay - 0.02 in/hr
 Rain Barrel/Cistern - 0.00 in/hr

Use Random Generation to Account for Infiltration Rate Uncertainty

Select Particle Size File: C:\Program Files\WinSLAMM\LOW\CPZ

Refresh Schematic
Delete
Cancel
Continue

The required broad crested weir outlet (the weir length is the downslope edge of the building):

Broad Crested Weir Biofilter Outlet

Land Use: Residential
Source Area: Roofs 1
Biofiltration Device Number 1
Outlet Number 1

A. Weir Crest Length (ft)

B. Weir Crest Width (ft)

C. Height from datum to bottom of weir opening (ft)

D. Check to use Default Weir Coefficients

Or Enter Weir Coefficient (English Units)

Cancel Continue Delete

Evapotranspiration and plant information (see ET calculation paper for typical soil and plant information; as noted earlier, the crop factors are likely larger for these harsh conditions compared to reported agricultural data).

Evapotranspiration

Land Use: Residential
Source Area: Roofs 1

Evapotranspiration Rate (in/day)

January	0.04
February	0.05
March	0.07
April	0.10
May	0.16
June	0.19
July	0.19
August	0.16
September	0.14
October	0.10
November	0.06
December	0.04

Biofiltration Device Number 1
Outlet Number 2

Area of Biofilter that is Vegetated (sf): 1634
Root Depth (ft): 0.5

Soil Porosity (Saturation Moisture Content, 0-1): 0.35
Soil Field Moisture Capacity (Fraction, 0-1): 0.34
Permanent Wilting Point (Fraction, 0-1): 0.04
ET Adjustment Factor for Actual Crop (decimal): 1.2

Supplemental Irrigation Used? Yes No

Fraction of available capacity when irrigation begins (0-1):
 Fraction of available capacity when irrigation stops (0-1):

Available capacity is the difference between the field capacity and the wilting point

Runoff volume results showing complete runoff reduction for the green roofs. The substrate or storage volumes can be reduced, then re-analyzed.

WinSLAMM Model Output

Runoff Volume

Data File: class examples green roofs.DAT
 Rain File: WinRing - Modcon v1 1981.RAIN
 Date: 11/01/09 Time: 06:01:12
 Site Description: SLO/SILT-MDR/NA-Medium Density Residential, No Alleys, In-Curb & Gutter

Runoff Volume [cu ft]		Source Area Runoff Volume Contribution				Pollutants				Output Summary		
Residential Areas - Runoff Volume [cu ft]	Roofs 1	Roofs 2	Paved Parking/Storage 1	Driveways 1	Driveways 2	Sidewalks/Walks 1	Sidewalks/Walks 2	Sheet Area 1	Sheet Area 2	Str Area		
Minimum:	0	0	1,000	0	0	0	0	0	0	0		
Maximum:	2,959	0	19,744	1,722	4,929	9,679	2,068	32,953	6,886			
Average:	0.29	0	645.4	155.9	4905	116.9	979.2	67.63	3,095	6765		
Total:	32,10	0	69,698	1,6821	5,38337	1,2611	1,05749	7,304	35,6886	7,30598	1	

Total Area, with Drainage and Outfall Controls - Runoff Volume [cu ft]

Number of Rain:	Rain Total (inches)	Total Below Drainage System	Total Above Drainage System	Total Above Outfall Controls	Piv	Total Losses (in)	Total Calculated CN	Peak Reduction Factor	Peak Flushing Rate	Pre-Ru Volun
Minimum:	0.01	0.6454	0.6454	0.6454	0.00	0.01	N/A			
Maximum:	2.96	31,4531	31,4531	31,4531	0.33	1.72	99.2			
Average:	0.30	21,903	21,903	21,903	0.20	0.23	95.5			
Total:	32,10	2.366E+06	2.366E+06	2.366E+06		25.56				



Ancient temple site at top of hill that had roof runoff cistern, Kamiros, Rhodes (ancient Greece, 7th century BC)

The homes of important officials had water delivered through clay pipes



Ancient clay pipe at Kamiros, Rhodes (ancient Greece, 7th century BC)

Regular citizens had to hand carry water from the cistern at the top of the hill back down to their homes



Cistern tank, Kamiros, Rhodes, collected roof runoff from adjacent temple located at top of the hill.



Steps alongside cistern allowing jugs to be filled from holes in wall to cistern.

Beneficial use of stormwater as a local resource needs to be seriously considered



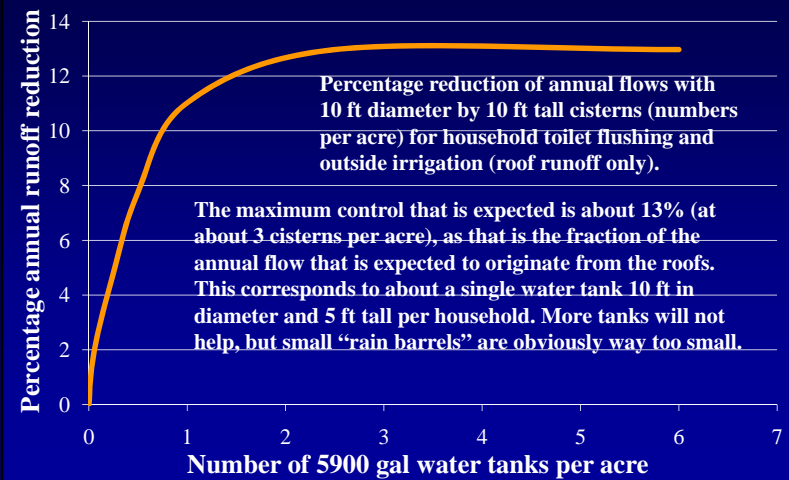
Prince George's County photo

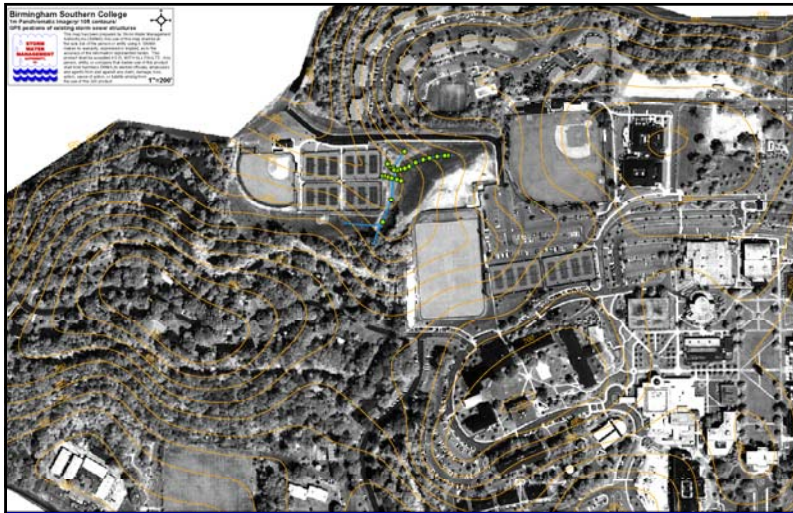
The water tank cisterns modeled for the Kansas City area were about 10 ft in diameter and 10 ft tall. The expected per household water use (gallons/day) from cisterns for toilet flushing and outside irrigation (ET deficit only) for the KC study area is:

January	113 gal/day	July	428
February	243	August	479
March	126	September	211
April	175	October	71
May	149	November	71
June	248	December	71



Number of water tanks and annual flow volume reductions for Kansas City test area





Birmingham Southern College Campus
 (map by Jefferson County Stormwater Management Authority)

Birmingham Southern College Fraternity Row (new construction at existing site)

	Acres	% of Total
Roadways	0.24	6.6%
Parking	0.89	24.5
Walks	0.25	6.9
Roofs	0.58	16.0
Landscaping	1.67	46.0
Total:	3.63	100.0

Supplemental Irrigation

	Inches per month (example)	Average Use for 1/2 acre (gal/day)
Late Fall and Winter (Nov-March)	1 to 1-1/2	230 - 340
Spring (April-May)	2 to 3	460 - 680
Summer (June-August)	4	910
Fall (Sept-Oct)	2 to 3	460 - 680
Total:	28 (added to 54 inches of rain)	

Capture and Reuse of Roof Runoff for Supplemental Irrigation

Tankage Volume (ft ³) per 4,000 ft ² Building	Percentage of Annual Roof Runoff used for Irrigation
1,000	56%
2,000	56
4,000	74
8,000	90
16,000	98

Modeling Beneficial Uses with a Cistern

- In this example, the beneficial uses of the roof runoff are toilet flushing and irrigation of the lawns and gardens surrounding the homes.
- The toilet flushing use was determined to be 71 gal/day per household based on regional census and water use data.
- Irrigation was calculated to supply the deficit between the monthly ET and rainfall values for turf grass. Additional discharges to the landscaping is possible if enhanced infiltration is desired.
- A simple tank 10 ft in diameter and 5 ft tall per house was used in this example.
- The cistern/rain barrel option was selected under the native soil infiltration rates (this turns all infiltration off, obviously!).

Cistern modeling using the biofiltration control option:

The broad crested weir is always required to provide an overflow from the tank. In actual cases, overflows will be through an overflow pipe. In this example, the weir length is the circumference of the tank. The total tank height must extend several inches above the bottom of the weir opening.

Biofilter Cistern/Rain Barrel

Land Use: Residential
Source Area: Roofs 1
Biofiltration Device Number 1
Outlet Number 2

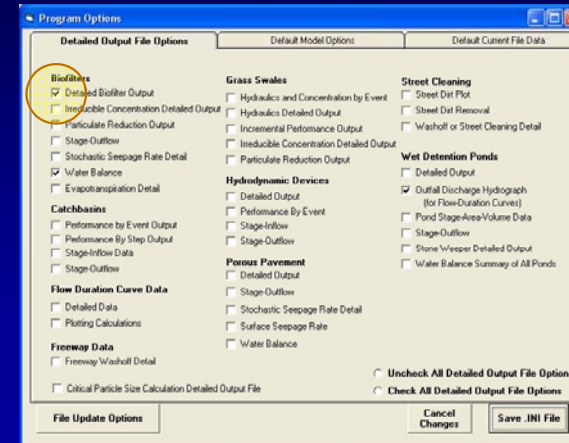
Month	Water Use Rate (gal/day)
January	113.00
February	243.00
March	126.00
April	175.00
May	149.00
June	248.00
July	428.00
August	479.00
September	211.00
October	71.00
November	71.00
December	71.00

These are the monthly calculated water use rates, based on household population and water use for toilet flushing, and the monthly average deficits between the ET and rainfall. Again, additional irrigation is possible if enhanced infiltration is desired.

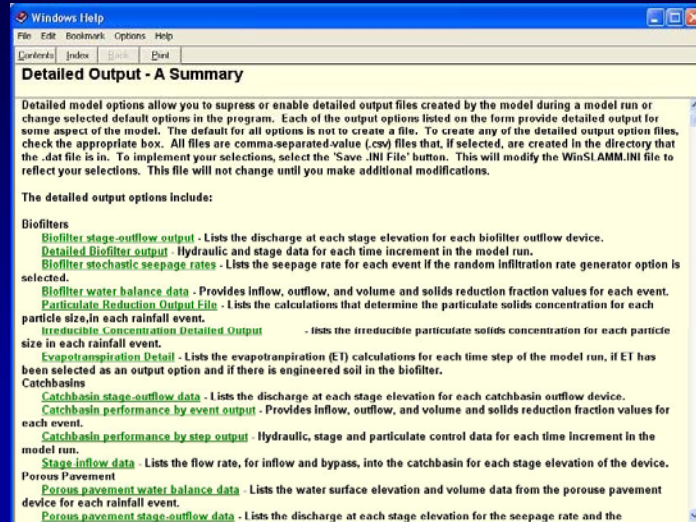
The cisterns at the homes associated with the directly connected roofs (roof 1 category) were able to use all of the roof runoff for toilet flushing and irrigation in this example.

Residential Areas - Runoff Volume (cu ft)		Roofs 1	Roofs 2	Paved Parking/Storage 1	Driveways 1	Driveways 2	Sidewalks/Walks 1	Sidewalks/Walks 2	Street Area 1	Street Area 2	St Area
Minimum:	0.01	0	0	1,000	0	0	0	0	0	0	0
Maximum:	2.93	0	19744	1722	49269	2673	9678	2068	20553	63825	
Average:	0.29	0	646.4	159.6	4895	116.8	979.2	67.63	3296	6785	
Total:	32.10	0	69698	16921	536337	12611	105749	7304	356886	730590	1

In WinSLAMM, there are many additional detailed output options that can be accessed with the program options menu. The detailed biofilter output allows one to see how much water is in the cistern at any time, amongst much other information:



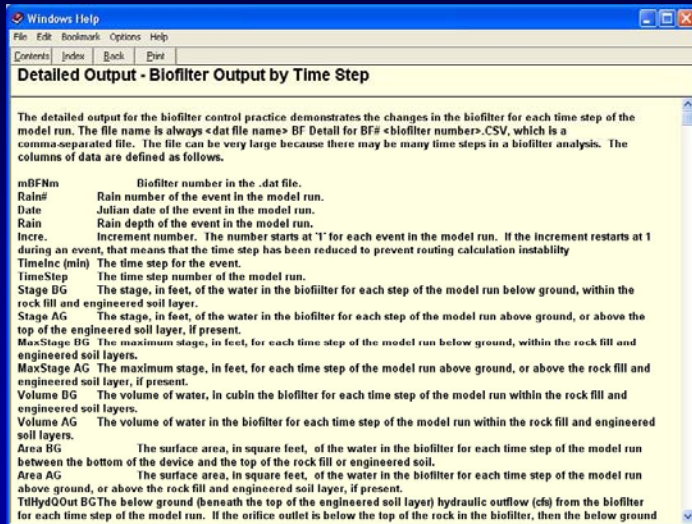
The help files describe the information provided in each output option. Selecting an output high-light expands the descriptions.



The following is an example of the detailed biofilter output, showing the details for each time step in the calculations (every 6 minutes in this example). The depth of the water in the cistern (stage above ground) is high-lighted below for a 2.59 inch rain:

Time (min)	Step	Time (days)	Stage AG (ft)	Stage BG (ft)	Stage AG (ft)	Stage BG (ft)	MaxStage	Volume	Volume	Area BG	Area AG	Eng Soil	AG Out	AG	AG	
8993	1	46	10758.21	2.59	657	6	187.6493	0.2962002	0.393	0	234.552	0	79	W/ALSEK	0	0
8994	1	46	10758.21	2.59	658	6	187.6534	0.2962209	0.393	0	234.413	0	79	W/ALSEK	0	0
8995	1	46	10758.21	2.59	659	6	187.6576	0.2962415	0.393	0	234.275	0	79	W/ALSEK	0	0
8996	1	46	10758.21	2.59	660	6	187.6618	0.2962621	0.393	0	234.137	0	79	W/ALSEK	0	0
8997	1	46	10758.21	2.59	661	6	187.6659	0.2962827	0.393	0	233.999	0	79	W/ALSEK	0	0
8998	1	46	10758.21	2.59	662	6	187.6701	0.2963033	0.393	0	233.861	0	79	W/ALSEK	0	0
8999	1	46	10758.21	2.59	663	6	187.6743	0.2963239	0.393	0	233.723	0	79	W/ALSEK	0	0
9000	1	46	10758.21	2.59	664	6	187.6784	0.2963445	0.393	0	233.585	0	79	W/ALSEK	0	0
9001	1	46	10758.21	2.59	665	6	187.6826	0.2963651	0.393	0	233.446	0	79	W/ALSEK	0	0
9002	1	46	10758.21	2.59	666	6	187.6868	0.2963857	0.393	0	233.308	0	79	W/ALSEK	0	0
9003	1	46	10758.21	2.59	667	6	187.6909	0.2964063	0.393	0	233.17	0	79	W/ALSEK	0	0
9004	1	46	10758.21	2.59	668	6	187.6951	0.2964269	0.393	0	233.032	0	79	W/ALSEK	0	0
9005	1	46	10758.21	2.59	669	6	187.6992	0.2964475	0.393	0	232.894	0	79	W/ALSEK	0	0
9006	1	46	10758.21	2.59	670	6	187.7034	0.2964681	0.393	0	232.756	0	79	W/ALSEK	0	0
9007	1	46	10758.21	2.59	671	6	187.7076	0.2964887	0.393	0	232.617	0	79	W/ALSEK	0	0
9008	1	46	10758.21	2.59	672	6	187.7117	0.2965093	0.393	0	232.479	0	79	W/ALSEK	0	0
9009	1	46	10758.21	2.59	673	6	187.7159	0.2965299	0.393	0	232.341	0	79	W/ALSEK	0	0
9010	1	46	10758.21	2.59	674	6	187.7201	0.2965505	0.393	0	232.203	0	79	W/ALSEK	0	0
9011	1	46	10758.21	2.59	675	6	187.7242	0.2965711	0.393	0	232.065	0	79	W/ALSEK	0	0
9012	1	46	10758.21	2.59	676	6	187.7284	0.2965917	0.393	0	231.927	0	79	W/ALSEK	0	0
9013	1	46	10758.21	2.59	677	6	187.7326	0.2966123	0.393	0	231.789	0	79	W/ALSEK	0	0
9014	1	46	10758.21	2.59	678	6	187.7367	0.2966329	0.393	0	231.651	0	79	W/ALSEK	0	0
9015	1	46	10758.21	2.59	679	6	187.7409	0.2966534	0.393	0	231.513	0	79	W/ALSEK	0	0
9016	1	46	10758.21	2.59	680	6	187.7451	0.296674	0.393	0	231.374	0	79	W/ALSEK	0	0
9017	1	46	10758.21	2.59	681	6	187.7492	0.2966946	0.393	0	231.236	0	79	W/ALSEK	0	0
9018	1	46	10758.21	2.59	682	6	187.7534	0.2967152	0.393	0	231.098	0	79	W/ALSEK	0	0

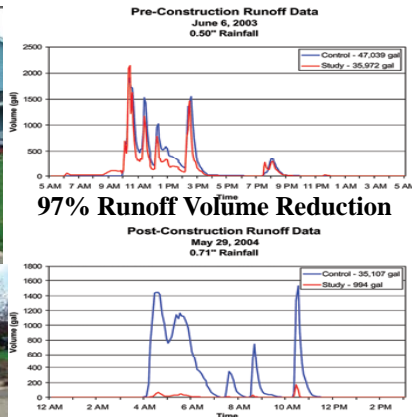
The context-sensitive help files explain each column header:



Rain Garden Designed for Complete Infiltration of Roof Runoff



Burnsville, Minnesota, Rainwater Gardens



An example of the dramatic runoff volume reductions possible through the use of conservation design principles (17 rain gardens, at about \$3,000 each, at 14 homes in one neighborhood)

Land and Water, Sept/Oct. 2004

Rain Garden Modeling

- Rain gardens are also modeled using the biofilter option.
- Rain gardens are usually very simple devices, constructed by excavating a 1 to 3 feet deep hole near the downspout of a building. The depth and area for the rain garden is usually determined by landscaping considerations, but should also be based on the soil infiltration rate and the roof area.
- The excavated hole is then partially back-filled with an amended soil. Native soils are not usually recommended due to the likely presence of clays. Most states now recommend a mixture of sand and an organic amendment. The excavated soil can be used to build up the downslope edge of the area to provide additional storage.
- Surface storage is also needed above the amended soil; a depression of at least several inches for a small rain garden to about a foot for a larger device is needed.
- Rain gardens are also usually planted with deep-rooted native plants to help enhance infiltration.

Input screen for a simple rain garden (2 per house in this example) in an area having marginal silt loam soils (0.3 inches/hr infiltration rate):

Biofiltration Control Device

Land Use: Residential
Source Area: Roofs 1
Total Area: 4.5 acres
Biofilter Number 1

Device Properties

Top Area (sf) 100
Bottom Area (sf) 75
Total Depth (ft) 2.00
Typical Width (ft) (Cost est. only) 10.00
Native Soil Infiltration Rate (in/hr) 0.300
Infiltration Rate Fraction (0-1) 1.00
Infiltration Rate Fraction Sides (0-1) 1.00
Rock Filled Depth (ft) 0.00
Rock Fill Porosity (0-1) 0.00
Engineered Soil Type Compost/Sand
Engineered Soil Infiltration Rate (in/hr) 2.10
Engineered Soil Depth (ft) 1.00
Engineered Soil Porosity (0-1) 0.30
Percent solids reduction due to Engineered Soil (0-100) N/A
Inflow Hydrograph Peak to Average Flow Ratio 3.80
Number of Devices in Source Area or Land Use 240

Add Outlet/ Discharge

1. Shallow Crested Weir
2. Broad Crested Weir
3. Vertical Stand Pipe
4. Evaporation
5. Rain Barrel/Cistern
6. Underdrain Outlet
7. Evapotranspiration

Edit Existing Outlet

Selected Outlets

1 - Broad Crested Weir

Change Geometry

Copy Biofilter Data Paste Biofilter Data

Select Native Soil Infiltration Rate

Sand - 8 in/hr
Loamy sand - 2.5 in/hr
Sandy loam - 1.0 in/hr
Loam - 0.5 in/hr
Silt loam - 0.3 in/hr
Sandy silt loam - 0.2 in/hr

Clay loam - 0.1 in/hr
Silty clay loam - 0.05 in/hr
Sandy clay - 0.05 in/hr
Silty clay - 0.04 in/hr
Clay - 0.02 in/hr
Rain Barrel/Cistern - 0.00 in/hr

Use Random Number Generation to Account for Infiltration Rate Uncertainty

Select Particle Size File C:\Program Files\WinSLAMM\LOW.CPZ

Source Areas from Land Use that Contribute Runoff to Biofiltration Control Device(s)

Roof top 1-5
Paved Parking/Storage 1-3
Unpaved Paving/Storage 1-2
Playground 1-2
Driveways 1-2
Sidewalks/Walks 1-2
Sheet Area 1-2
Paved Land and Shoulder 1-5
Large Landscaped Area 1
Small Landscaped Area 1-2
Other Previous Area
Other Dr. Crotch Imp Area
Large Tall Area
Undeveloped Areas
Other Previous Areas
Other Directly Cont'd Imp
Other Partially Cont'd Imp

Biofilter Geometry Schematic

Refresh Schematic Delete Cancel Continue

The required broad-crested weir is just the downslope overflow, the only other water discharge is infiltration into the native soil. With marginal soils, greater surface storage is needed:

Broad Crested Weir Biofilter Outlet

Land Use: Residential
Source Area: Roofs 1
Biofiltration Device Number 1
Outlet Number 1

A. Weir Crest Length (ft) 10
B. Weir Crest Width (ft) 0.5
C. Height from datum to bottom of weir opening (ft) 1.75
D. Check to use Default Weir Coefficients

Or Enter Weir Coefficient (English Units)

Cancel Continue Delete

Reduced runoff occurs from the roof areas having rain gardens. In this example, overflows occurred several times a year during the largest storms (under the "view" drop down menu, select output option 1 to see results for each rain individually). Only 13,000 ft³ was discharged from these roofs during this period, compared to over 120,000 ft³ with no controls. This could be reduced further, if desired, by using larger, or more, rain gardens, and re-analyzed using the model.

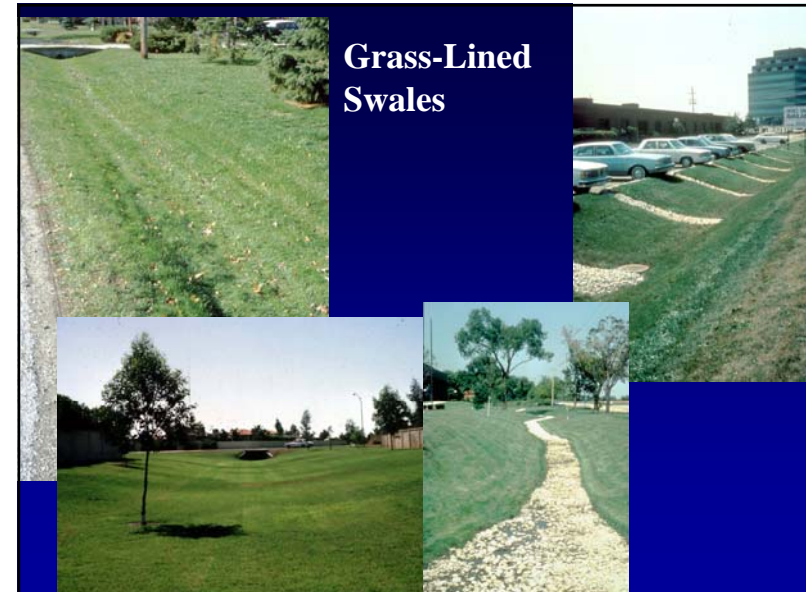
WinSLAMM Model Output

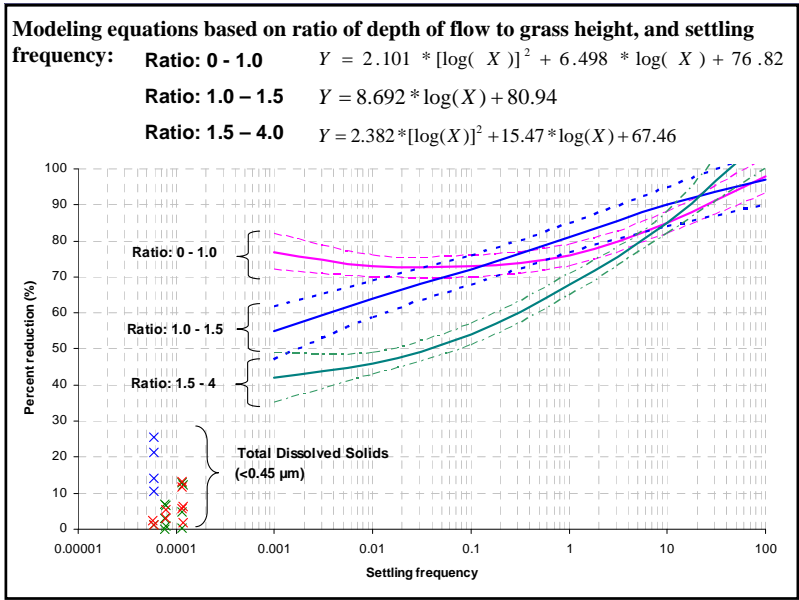
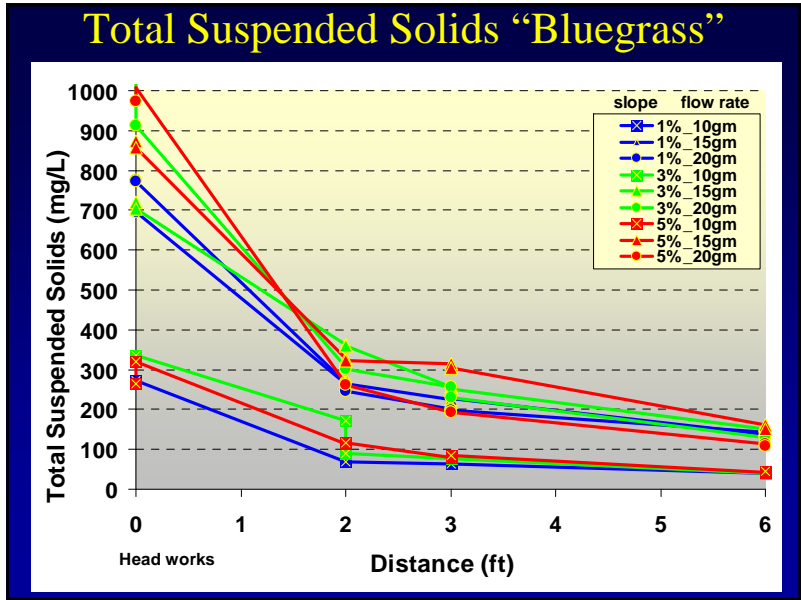
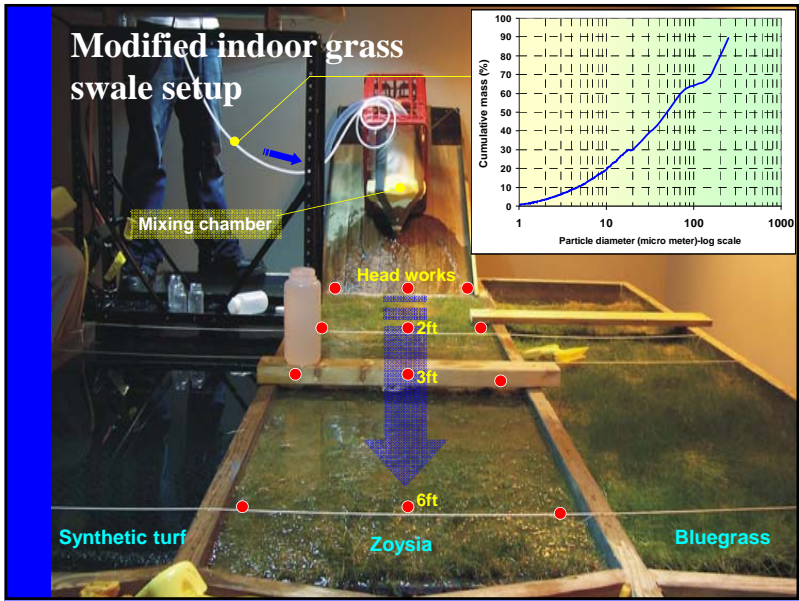
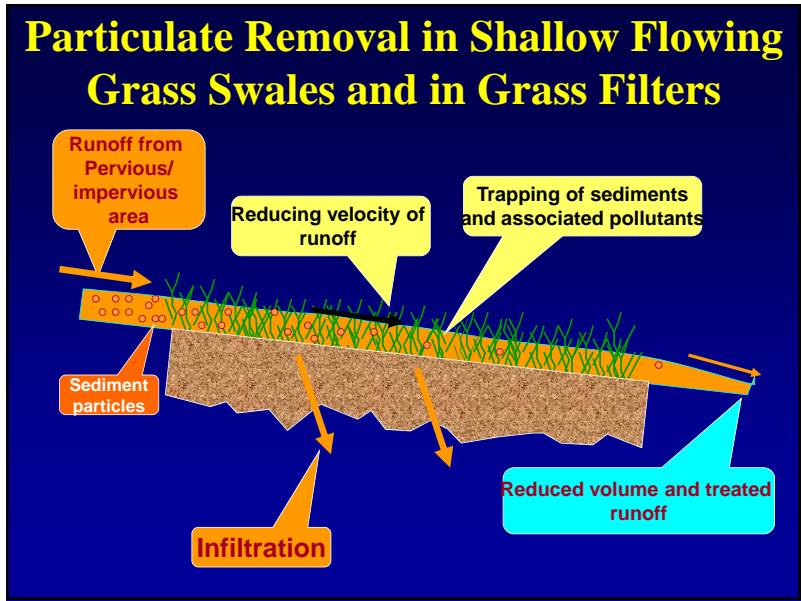
Runoff Volume

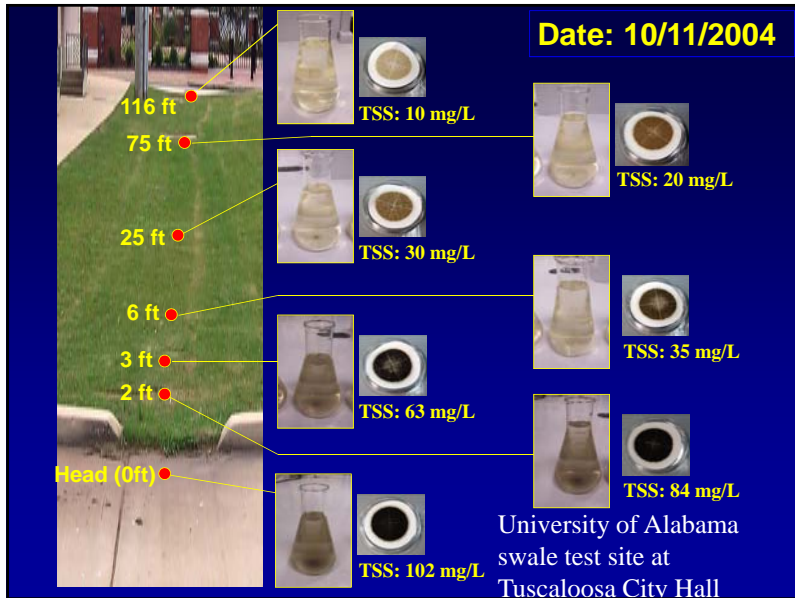
Data File: class examples\rain garden\dat
Rain File: WinReg - Madison Vt 1981 RAN
Date: 11-01-09 Time: 06:21:51
Site Description: SLD/SIL1 and PPA-Medium Density Residential, No Alleys, 1st Curb & Gutter

Residential Areas - Runoff Volume (cu ft)	Roofs 1	Roofs 2	Paved Parking/Storage 1	Driveways 1	Driveways 2	Sidewalks/Walks 1	Sidewalks/Walks 2	Sheet Area 1	Sheet Area 2	St Area
Summery for All Events	0	0	1,000	0	0	0	0	0	0	0
Minimum	0.01	0	0	0	0	0	0	0	0	0
Maximum	2.99	11050	19744	1722	49293	3973	9678	2088	32993	68885
Average	0.29	1204	6454	155.8	4365	116.8	579.2	67.63	3295	6765
Total	32.19	1300	63698	16621	538227	12611	105749	7304	255896	730590

Total Area, with Drainage and Outlet Controls - Runoff Volume (cu ft)	Run Total (inches)	Total Before Drainage Systems	Total After Drainage Systems	Total After Outlet Controls	Rv	Total Losses (in)	Calculated CN	Peak Reduction Factor	Flushing Ratio	Pre-Ru Value
Number of Runs	108	108	108	108						
Minimum	0.01	0.6454	0.6454	0.6454	0.00	0.01	N/A			
Maximum	2.99	325581	325581	325581	0.35	1.69	99.2			
Average	0.30	22024	22024	22024	0.20	0.23	95.5			
Total	32.19	2.379E+06	2.379E+06	2.379E+06		25.63				







Modeling Grass Swales

- The main input screen for grass swales is under the “land use” drop down menu under the “catchbasin or drainage control” option, and then select “drainage control.”
- In this example, the swale density is selected based on the land use. The total length of the swales is automatically calculated, along with the average swale length to the outfall (based on the total service area).
- The swale bottom width, side slope, and longitudinal slope are entered based on the area to be served. Residential areas usually have relatively narrow swale widths, while industrial areas have larger widths. These dimensions are determined in concert with the drainage design requirements for the area.
- The swale grass retardance factor is from USDA research and is usually D for urban areas. The grass height is a function of maintenance, but 3 inches may be suitable.
- The swale dynamic infiltration rate is about half of the normal infiltration rate for most swales, but is equal to the soil infiltration rate for relatively flat swales. See the help files for more information.

Main grass swale input screen:

Grass Swale Data	Combined Land Uses	Residential Land Use	Institutional Land Use	Commercial Land Use	Industrial Land Use	Other Urban Land Use	Freeway Land Use
Total Area in Land Use (ac)		100.00					
Area Served by Swales (ac)		100.00					
Swale Density (#/ac)		250.00					
Average Swale Length to Outlet (ft)		31.21					
Typical Bottom Width (ft)		3.0					
Typical Swale Side Slope (H:1:V)		3.0					
Typical Longitudinal Slope (ft/V, V/H)		0.010					
Swale Retardance Factor		D					
Typical Grass Height (in)		3.0					
Swale Dynamic Infiltration Rate (in/hr)		0.150					
Typical Swale Depth (ft) for Cost Analysis (Optional)		3.0					

Context-sensitive grass swale help file:

The grass swale control device allows the user to determine the pollutant reduction and runoff volume reduction due to grass swales. The model determines the runoff volume reduction by calculating the infiltration loss for each time step. The particulate reduction is based upon the settling frequency of the particles entering the swales and the height of the grass relative to the flow depth. The grass swale filters the runoff using the settling frequency and the length of the swale. The algorithms used to determine the Mannings n values were developed from the masters thesis work by Jason Kirby (Kirby, J.T., S.R. Durrans, R. Pitt, and P.D. Johnson, "Hydraulic resistance in grass swales designed for small flow conveyance," *Journal of Hydraulic Engineering*, Vol. 131, No. 1, Jan. 2005) as part of a WERF-supported research project. Johnson, P.D., R. Pitt, S.R. Durrans, M. Umula, and S. Clark. *Metals Removal Technologies for Urban Stormwater*. Water Environment Research Foundation, WERF 97-IRM-2. ISBN: 1-94339-682-3. Alexandria, VA. 701 pgs. Oct. 2003. The particulate trapping algorithms were based on the masters thesis research conducted by Yukio Nara (Nara, Y., R. Pitt, S.R. Durrans, and J. Kirby, "Sediment transport in grass swales," in *Stormwater and Urban Water Systems Modeling*, Monograph 14, edited by W. James, K.W. Irvine, E.A. McBean, and R.E. Pitt. CH. Guelph, Ontario, pp. 379-402, 2006), supported by the University Transportation Center for Alabama. "Alabama Highway Drainage Conservation Design Practices - Particulate Transport in Grass Swales and Grass Filters", by Yukio Nara and Robert Pitt, University Transportation Center for Alabama, University of Alabama, Tuscaloosa, Alabama, November, 2005.

Grass swale performance is determined by routing a complex triangular hydrograph through the swales entered in the model by the user. Runoff volume reductions are determined by infiltration losses, and particulate losses are determined through particle trapping.

Runoff volume is reduced by the dynamic infiltration rate of the swales for each six minute time step of the hydrograph. The flow and the swale geometry are used to determine the Mannings n to iteratively determine the depth of flow in the swale for each time step, using traditional VR-n curves that were extended by Kirby to cover the smaller flows found in roadside swales. Using the calculated depth of flow for each time increment, the model calculates the wetted perimeter (based on the swale cross-sectional shape) which is then multiplied by the total swale length to determine the area used to infiltrate the runoff. Detailed for these calculations are available by selecting the "Hydraulics Detailed Output File" checkbox from the "Detailed Output Options" listing under "Program Options". The event by event summary detailed output is available by selecting the "Hydraulics and Concentration by Event" checkbox from the "Detailed Output Options" listing. These comma-separated tabular files are created when the model is executed and can be reviewed using a spreadsheet after importing the files.

Particulate filtering is calculated for each time step using the average swale length to the outlet and the calculated depth of flow for each 6-minute time step of the hydrograph. The depth of flow and swale geometry are used to calculate the flow velocity, which in turn is used to determine the travel time, settling velocity, and settling frequency for the average swale length in the study area. This information is used to determine the flow depth to grass height ratio needed to calculate particulate trapping, adapted from the Nara and Pitt reference cited above. The settling frequency and resultant particulate trapping is calculated for each of the thirty-one particle size fractions in the selected particle size distribution file. The detailed output for these calculations is available by selecting the "Particulate Reduction Detailed Output File" checkbox from the "Detailed Output Options" listing under "Program Options". These comma-separated tabular files are created when the model is executed and can be reviewed using a spreadsheet after importing the files.

Output summary after grass swale analysis. The difference between the “total before drainage system” and “total after drainage system” shows the effects of the swales. In this example, about 47% runoff volume and 53% particulate solid discharge reductions were calculated:

Drainage System and Outfall Output Summary	Runoff Volume (cu ft)	Percent Runoff Reduction (%)	Runoff Coefficient (Cv)	Particulate Solids Conc. (mg/L)	Particulate Solids Yield (lb)	Percent Particulate Solids Reduction (%)
Source Area Total without Controls	2.843E+06	0.00%	0.24	137.4	24369	
Outfall Total without Controls	2.843E+06	0.00%	0.24	137.4	24369	
Current File Output - Total Before Drainage System	2.843E+06	0.00%	0.24	137.4	24369	
Current File Output - Total After Drainage System	1.501E+06	47.20%	0.13	122.2	11456	53.00%
Current File Output - Total After Outfall Controls	1.501E+06	47.20%	0.13	122.2	11456	53.00%
Current File Output - Annualized Total After Outfall Controls	1.505E+06				11489	
Total Area Modeled (ac)	100.00					
Years in Model Run	1.00					

Total Control Practice Costs	Value
Capital Cost	N/A
Land Cost	N/A
Annual Maintenance Cost	N/A
Present Value of All Costs	N/A
Annualized Value of All Costs	N/A

Receiving Water Impacts Due To Stormwater Runoff (DWP Impervious Cover Model)	Without Controls	With Controls	Approximate Urban Stream Classification
Calculated Rv	0.24	0.13	
	Poor	Fair	

In addition, the calculated urban receiving water classification (based on the Center for Watershed Protection's *Impervious Cover Model*) may improve from poor to fair, if the complete watershed was similarly developed, compared to development without controls.

The runoff volume detailed output also shows the benefits of the swale. This is the expanded output showing the effects for each individual rain, available by selecting the full output option 1 under the “view” drop down menu.

The screenshot shows the 'WinSWM Model Output' window with a detailed runoff volume table. The table is organized into four main sections: Runoff Volume, Particulate Solids, Pollutants, and Output Summary. The 'Runoff Volume' section is further divided into 'Runoff Volume (cu ft)' and 'Source Area Runoff Volume Contribution'. A yellow oval highlights the 'Total After Drainage System' column, which shows the volume of runoff after accounting for the swale's benefits.

Start Date	Rain Total (inches)	Total Before Drainage System	Total After Drainage System	Total After Outlet Controls	Riv	Total Losses (ft)	Calculated CN*	Peak Reduction Factor	Flushing Ratio	Det. Basin Out. Struct. Failed (cu ft)	Pie-Ru Volun
01/01/91	0.02	502.3	0	0	0.00	0.02	N/A				
01/06/91	0.06	2303	0	0	0.00	0.06	N/A				
01/06/91	0.01	11.02	0	0	0.00	0.01	N/A				
01/15/91	0.01	11.02	0	0	0.00	0.01	N/A				
01/31/91	0.11	5576	0	0	0.00	0.11	N/A				
02/05/91	0.05	1767	0	0	0.00	0.05	N/A				
02/06/91	0.06	2303	0	0	0.00	0.06	N/A				
02/06/91	0.01	11.02	0	0	0.00	0.01	N/A				
02/09/91	0.01	11.02	0	0	0.00	0.01	N/A				
02/10/91	0.30	2766	295.0	295.0	0.00	0.30	N/A				
02/21/91	1.50	152412	70490	70490	0.12	1.20	74.9				
02/23/91	0.06	2303	0	0	0.00	0.06	N/A				
02/27/91	0.24	15299	3.406	3.406	0.00	0.24	N/A				
03/10/91	0.06	2303	244.7	244.7	0.01	0.06	36.9				
03/25/91	0.07	2900	0	0	0.00	0.07	N/A				
03/29/91	0.05	1767	0	0	0.00	0.05	N/A				
03/29/91	0.05	2303	0	0	0.00	0.06	N/A				
03/29/91	0.07	2900	0	0	0.00	0.07	N/A				
04/03/91	0.02	176.7	0	0	0.00	0.02	N/A				
04/03/91	0.25	16311	4948	4948	0.05	0.25	82.9				
04/07/91	0.71	63117	21001	21001	0.08	0.65	64.0				
04/08/91	0.41	30627	9989	9989	0.07	0.30	90.0				
04/10/91	1.06	89562	80495	80495	0.21	0.94	95.0				
04/12/91	0.13	7486	3289	3289	0.07	0.12	96.7				
04/13/91	0.32	20095	5672	5672	0.05	0.30	91.3				
04/16/91	0.01	11.02	0	0	0.00	0.01	N/A				
04/19/91	0.04	1202	0	0	0.00	0.04	N/A				