

## Stormwater Non-potable Beneficial Uses: Modeling Groundwater Recharge at a Stormwater Drywell Installation

Leila Talebi<sup>1</sup> and Robert Pitt<sup>2</sup>

<sup>1</sup>Graduate student, Department of Civil, Construction, and Environmental Engineering, University of Alabama, [ltalebi@crimson.ua.edu](mailto:ltalebi@crimson.ua.edu)

<sup>2</sup>Cudworth Professor, Urban Water Systems, Department of Civil, Construction, and Environmental Engineering, University of Alabama, [rpitt@ceng.ua.edu](mailto:rpitt@ceng.ua.edu)

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## Introduction

- Besides the obvious benefit of reduced stormwater discharges (and attendant receiving water benefits), many stormwater use options also benefit other components of the urban water infrastructure.
- If stormwater is stored and used to irrigate landscaped areas and flush toilets, as is common in many water-stressed locations today, less highly treated domestic water needs to be delivered.
- Groundwater recharge to augment local groundwater resources, possibly for later local use as demonstrated in many developing countries, is also a suitable beneficial use of stormwater.

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## Millburn, NJ (Background)

- For the past several years, the city of Millburn has required dry wells/cisterns to infiltrate the increased flows from newly developed areas.
- There are some water storage tanks now being installed to use the increased stormwater for irrigation.
- The current project supported by the Wet Weather Flow Research Program of the US EPA is investigating whether increased beneficial uses of the runoff would be a more efficient use of the water instead of infiltrating into the shallow groundwaters.
- There are substantial data available for this community, and we are supplementing these data with more detailed site information to allow a comprehensive review of beneficial stormwater uses.

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- This EPA project in Millburn includes monitoring the water levels in several dry wells and concurrent rainfall conditions. This information is also being used to calibrate WinSLAMM for detailed evaluations of alternative stormwater management options, including beneficial water use (irrigation and groundwater recharge).
- This information, along with the rainfall and evapotranspiration data, will be used to calculate the amount of stormwater that can be beneficially used for local groundwater recharge and site irrigation and to show how landscaping irrigation needs integrates with the available rainfall.

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### Location of monitoring dry wells

|   | Address                               |
|---|---------------------------------------|
| A | 383 Wyoming Avenue, Millburn, NJ.     |
| B | 258 Main St., Millburn, NJ.           |
| C | 11 Fox Hill Ln, Millburn, NJ.         |
| D | 8 South Beechcroft Rd., Millburn, NJ. |
| E | 2 Undercliff Rd., Millburn, NJ.       |
| F | Linda's Flower, Millburn, NJ.         |
| G | 9 Lancer, Millburn, NJ.               |

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### Land Development Characteristics (a sample site)

| Category     | Percent |
|--------------|---------|
| Pitched roof | ~10     |
| Paved...     | ~5      |
| Drive way    | ~5      |
| Sidewalk     | ~2      |
| Street       | ~10     |
| Landscape    | ~65     |
| Other        | ~2      |

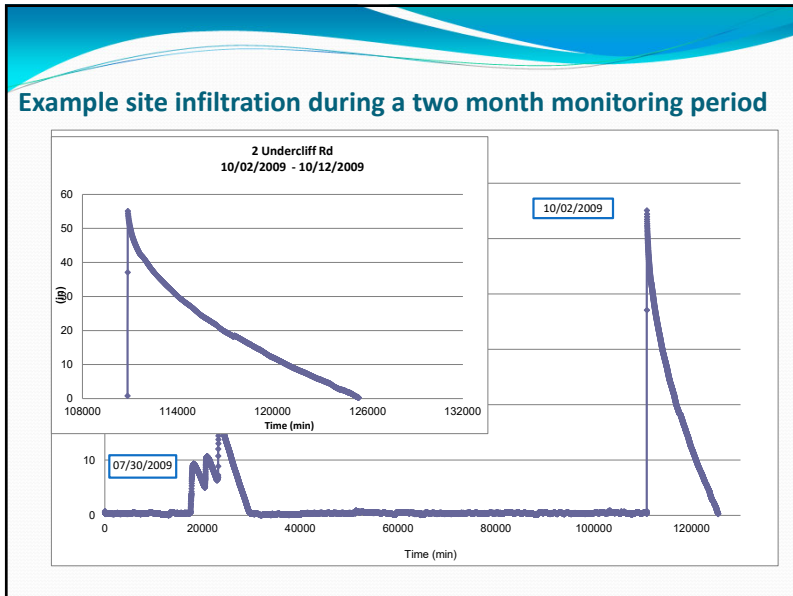
Most of areas covered by landscape (grass + trees)

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### Soil (sourc: online soil survey, USDA)

| Address               | Soil Name  | Soil Group | Slope (%) | K <sub>sat</sub>                                       | Drainage class | Typical profile   |
|-----------------------|--|------------|-----------|--|----------------|---|
| 383 Wyoming Ave.      | Boonton-Urban land, Boonton substratum complex, red sandstone      | C          | 3-8       | Moderately low to moderately high (0.06 to 0.20 in/hr) | Well drained   | 0-1 in: Slightly decomposed plant<br>1-3 in: Silt loam<br>3-10 in: Loam<br>10-27 in: Gravelly loam<br>27-67 in: Gravelly fine sandy loam<br>67-83 in: Gravelly sandy loam |
| 258 Main St.          | Dunellen sandy loam  | A          | 3-8       | High (1.98 to 5.95 in/hr)                              | Well drained   | 0-42 in: Sandy loam<br>42-70 in: Stratified gravelly sand to sand to loamy sand   |
| 11 Fox Hill Ln        | Boonton - Urban land, Boonton substratum complex, red sandstone    | C          | 3-8       | Moderately low to moderately high (0.06 to 0.20 in/hr) | Well drained   | 0-1 in: Highly decomposed plant<br>1-24 in: Sandy loam<br>24-42 in: Gravelly sandy loam<br>42-60 in: Fine sandy loam  |
| 8 South Beechcroft Rd | Boonton - Urban land, Boonton substratum complex, terminal moraine | C          | 3-8       | Moderately low to moderately high (0.06 to 0.20 in/hr) | Well drained   | 0-1 in: Highly decomposed plant<br>1-24 in: Sandy loam<br>24-42 in: Gravelly sandy loam<br>42-60 in: Fine sandy loam  |
| 2 Undercliff Rd       | Boonton - Urban land, Boonton substratum complex, terminal moraine | C          | 3-8       | Moderately low to moderately high (0.06 to 0.20 in/hr) | Well drained   | 0-1 in: Highly decomposed plant<br>1-24 in: Sandy loam<br>24-42 in: Gravelly sandy loam<br>42-60 in: Fine sandy loam  |
| Linda's Flower        | Boonton - Urban land, Boonton substratum complex, terminal moraine | C          | 3-8       | Moderately low to moderately high (0.06 to 0.20 in/hr) | Well drained   | 0-1 in: Highly decomposed plant<br>1-24 in: Sandy loam<br>24-42 in: Gravelly sandy loam<br>42-60 in: Fine sandy loam  |
| 9 Lancer              | Boonton - Urban land, Boonton substratum complex                   | C          | 8-15      | Moderately low to moderately high (0.06 to 0.20 in/hr) | Well drained   | 0-5 in: Loam<br>5-30 in: Silt loam<br>30-40 in: Gravelly fine sandy loam<br>40-47 in: Fine sandy loam<br>47-72 in: Loamy sand   |

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## Infiltration

- Horton (1940)

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$$f = f_c + (f_o - f_c)e^{-kt}$$

$f$ : the infiltration rate at time  $t$  (in/hr),  
 $f_o$ : the initial infiltration rate (in/hr),  
 $f_c$ : the final infiltration rate (in/hr),  
 $k$ : first-order rate constant ( $hr^{-1}$ ).

This equation assumes that the rainfall intensity is greater than the infiltration capacity at all times and that the infiltration rate decreases with time

- Green-Ampt (1911)

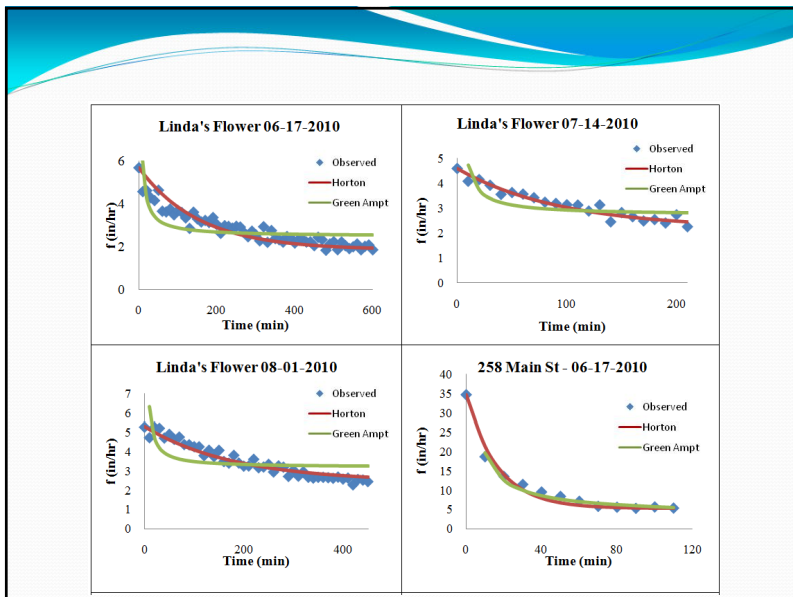
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$$f_t = K \left( \frac{\psi \Delta \theta}{F_t} + 1 \right)$$

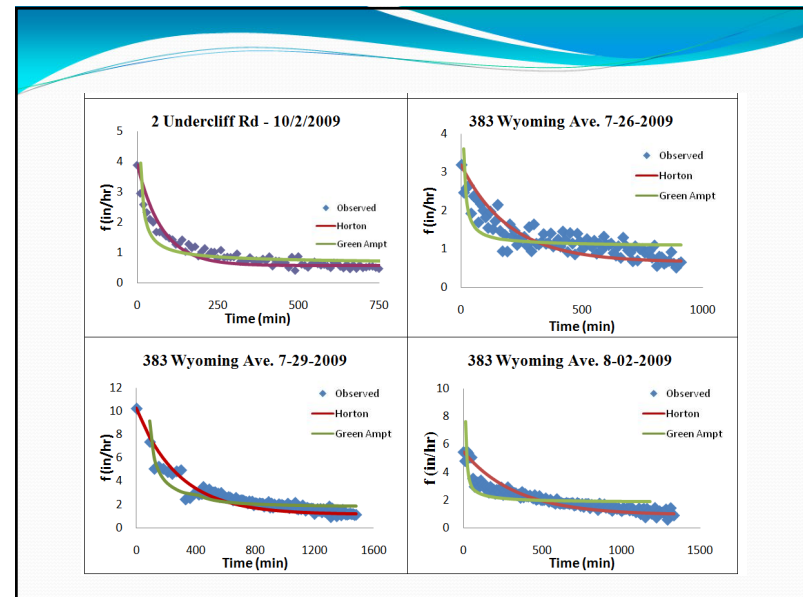
$f_t$ : infiltration rate, cm/hr;  
 $\psi$ : the initial Matric potential of the soil, in;  
 $\Delta \theta$ : the difference of soil water content after infiltration with initial water content,  $in^3/in^3$ ;  
 $K$ : hydraulic conductivity, in/hr;  
 $F_t$  is the cumulative infiltration at time  $t$ , in.

This equation requires a linear relationship between  $f_t$  and  $(1/F_t)$

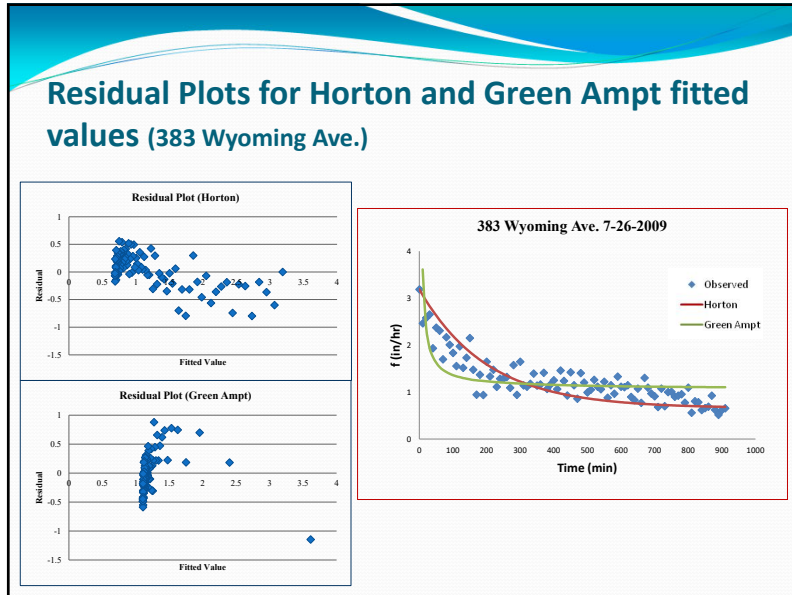
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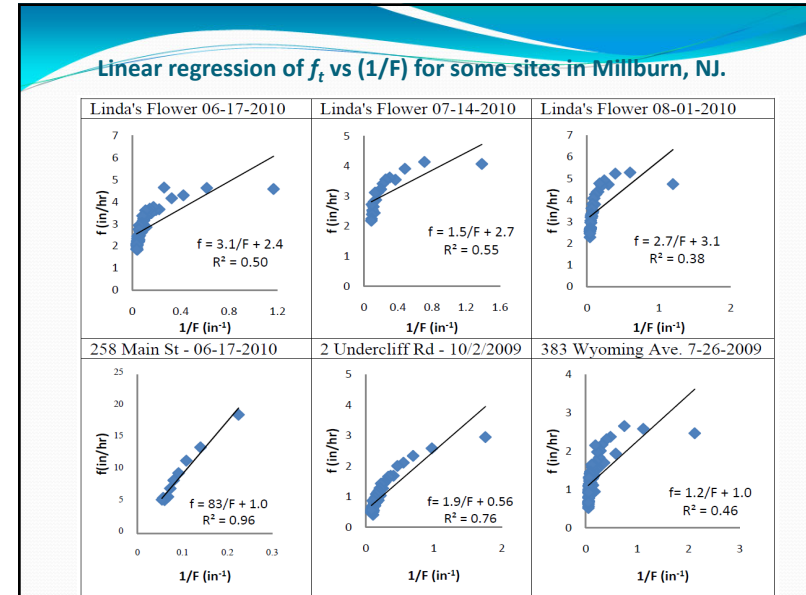
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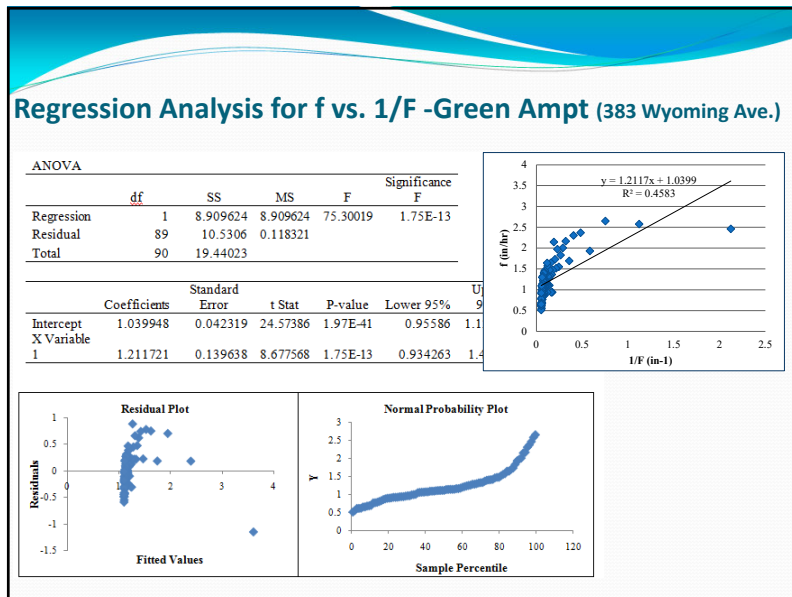
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### Horton's Parameters summary

| Site Address     | Soil Group | Date       | Horton's parameters (Millburn observed data) |               |           | Akan (1993) | Pitt et al. (1999)* |               |           |
|------------------|------------|------------|--|---------------|-----------|-------------|---------------------|---------------|-----------|
|                  |            |            | $f_0$ (in/hr)                                | $f_c$ (in/hr) | k (1/min) |             | $f_0$ (in/hr)       | $f_c$ (in/hr) | k (1/min) |
| Linda's Flower   | C          | 06-17-2010 | 5.7  | 1.9           | 0.0065    | 2-6         | 1.0-3.75            | 0.0-0.5       | 0.03-0.1  |
|                  |            | 07-14-2010 | 5.6  | 2.2           | 0.011     |             |                     |               |           |
|                  |            | 08-01-2010 | 5.3  | 2.5           | 0.0055    |             |                     |               |           |
| 258 Main St.     | A          | 06-17-2010 | 35   | 5.3           | 0.06      | 6-10        | 1.5-12              | 0.25-1.3      | 0.05-0.2  |
|                  |            | 07-14-2010 | 75   | 6.8           | 0.07      |             |                     |               |           |
|                  |            | 08-01-2010 | 75   | 4.7           | 0.045     |             |                     |               |           |
| 2 Undercliff     | C          | 10-02-2009 | 3.9  | 0.57          | 0.013     | 2-6         | 1.0-3.75            | 0.0-0.5       | 0.03-0.1  |
| 383 Wyoming Ave. | C          | 7-26-2009  | 3.2  | 0.66          | 0.005     | 2-6         | 1.0-3.75            | 0.0-0.5       | 0.03-0.1  |
|                  |            | 7-29-2009  | 10   | 1.1           | 0.0035    |             |                     |               |           |
|                  |            | 8-02-2009  | 5.5  | 0.93          | 0.003     |             |                     |               |           |
|                  |            | 8-22-2009  | 3.6  | 1.2           | 0.03      |             |                     |               |           |
|                  |            | 10-02-2009 | 5.6  | 1.2           | 0.0045    |             |                     |               |           |

\* 25<sup>th</sup> and 75<sup>th</sup> percentile values for compact sandy conditions

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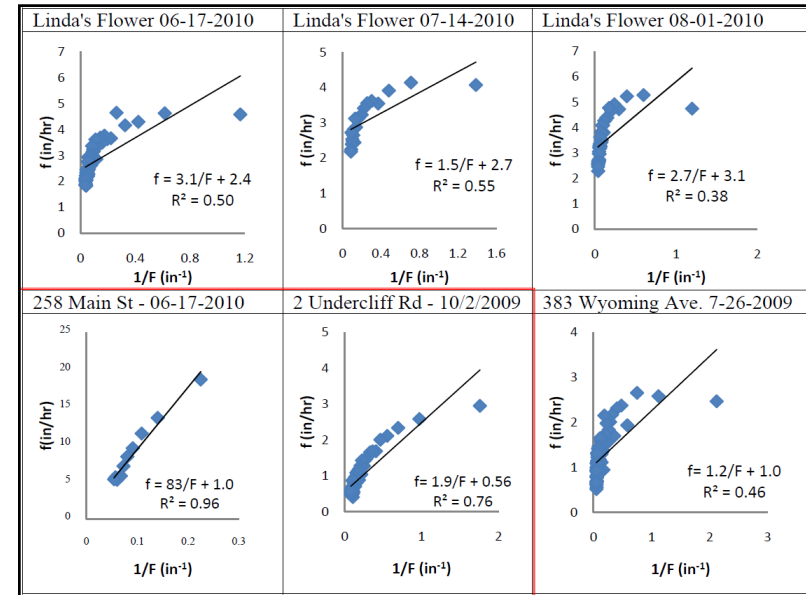


## Green-Ampt parameters

| Site Address     | Date       | Hydraulic conductivity K (in/hr) |                     |
|------------------|------------|----------------------------------|---------------------|
|                  |            | Millburn (case study)            | Rawls et al. (1983) |
| Linda's Flower   | 06-17-2010 | 2.435                            | 0.429               |
|                  | 07-14-2010 | 2.685                            |                     |
|                  | 08-01-2010 | 3.131                            |                     |
| 258 Main St.     | 06-17-2010 | 1.018                            | 1.17                |
| 2 Undercliff     | 10-02-2009 | 0.557                            | 0.429               |
| 383 Wyoming Ave. | 7-26-2009  | 1.039                            | 0.13-0.43           |

- For the rest of sites "K" values estimated from fitted curves were *negative*.

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## Conclusion

- The fitted graphs and resulting derived parameters of each mentioned equations indicate that although the fitted Horton curve is **visually** fitted better to observed data of the case study area than Green-Ampt curve, the **calculated parameters** of both used infiltration models **don't compare to the literature**.
- It is necessary to have **local measured data** for modeling and don't rely only on literature values.
- Later project activities will involve extensive modeling of stormwater beneficial use opportunities at many US locations, including Millburn, NJ with WinSLAMM model, resulting sustainable water management to make top-level evaluations of this water source.

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## Thank You

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