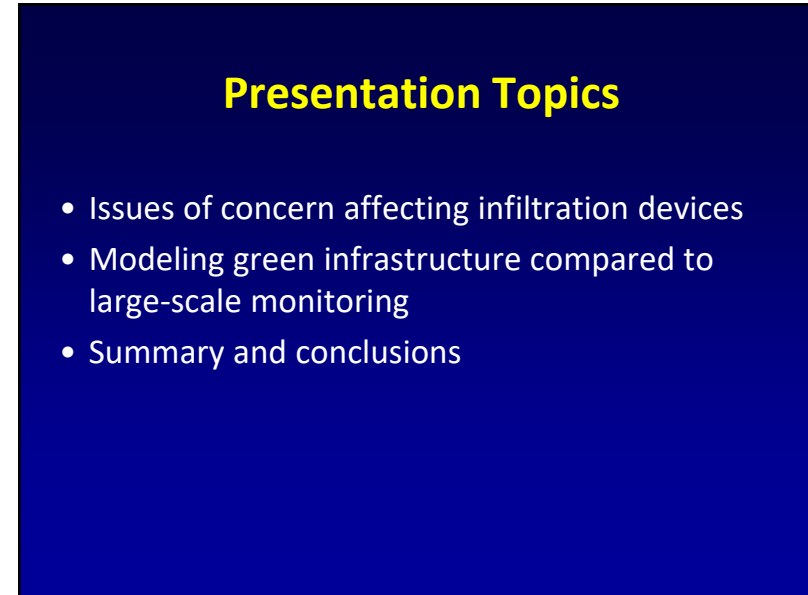
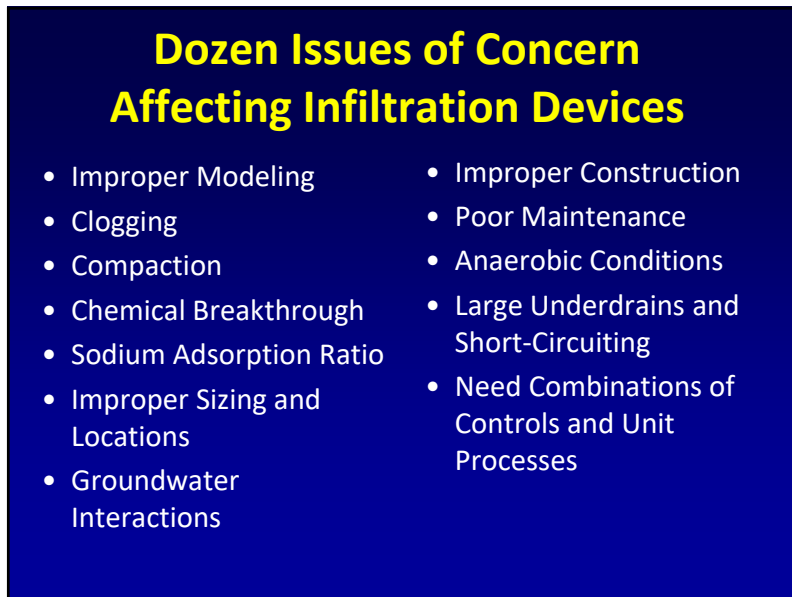


1



2



3



4

**Probability distribution of rains (by count) and runoff (by depth).**

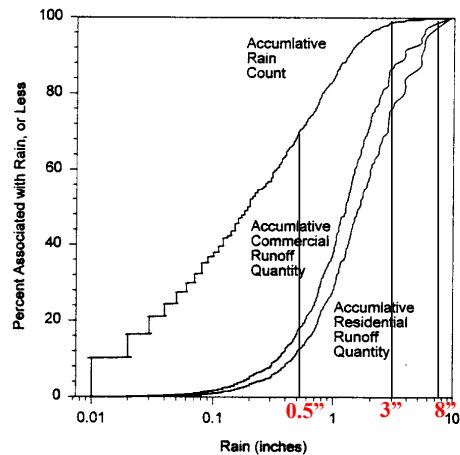
**Birmingham Rains:**  
 <0.5": 65% of rains  
 (10% of runoff)

0.5 to 3": 30% of rains  
 (75% of runoff)

3 to 8": 4% of rains  
 (13% of runoff)

>8": <0.1% of rains  
 (2% of runoff)

**Birmingham, AL Rain & Runoff Distributions ('81-'89)**

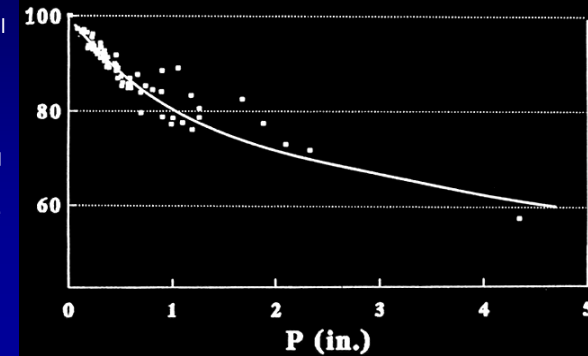


5

Typical plot of calculated curve numbers for actual site monitoring. This date is from the Univ. of Florida's rainfall-runoff database that contains historical SCS and COE monitoring data that was used to develop TR-55. Obviously, the CN method is only applicable for the large drainage design storms for which it was intended!

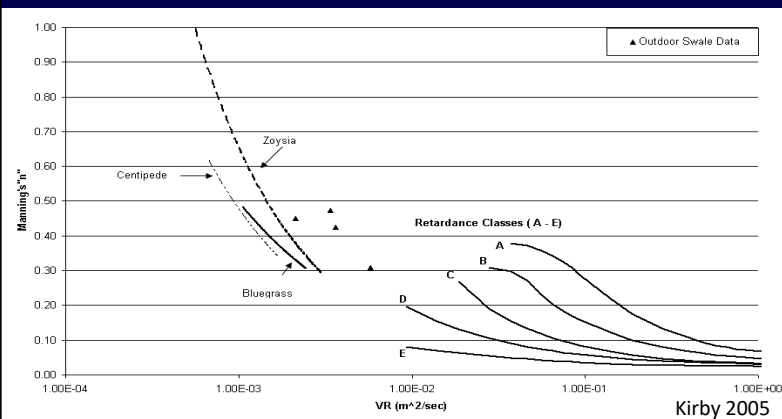
**Observed Curve Numbers for Residential Area (39% Imperviousness)**

**0.25 acre lots, sandy soils (CN for A soils: 61 and CN for B soils: 75)**



6

**Indoor vs. Historical Stillwater, Oklahoma, Retardance Curves**



From such graphs, swale hydraulic characteristics can be predicted on the basis of flow rate, cross sectional geometry, slope, and vegetation type. Small-scale urban drainage systems and flows are quite different from originally studied larger systems.

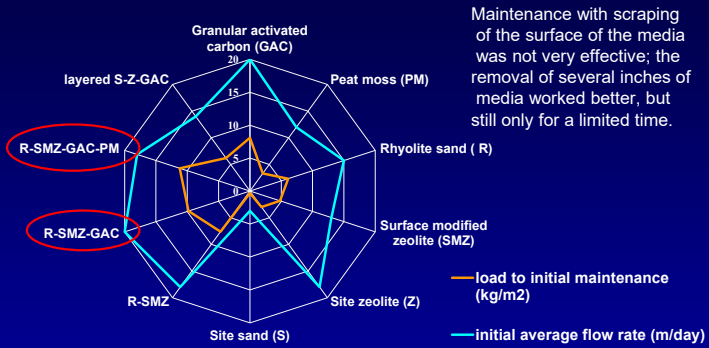
7

**2) Clogging**

- Clogging of biofiltration/bioretention devices hinders their long-term performance.
- Grass swales are relatively robust as they are very large in comparison to the service area and sediment load.
- Many smaller infiltration devices suffer due to excessive sediment without adequate pre-treatment.

8

### Want media with good clogging and good flow rate characteristics

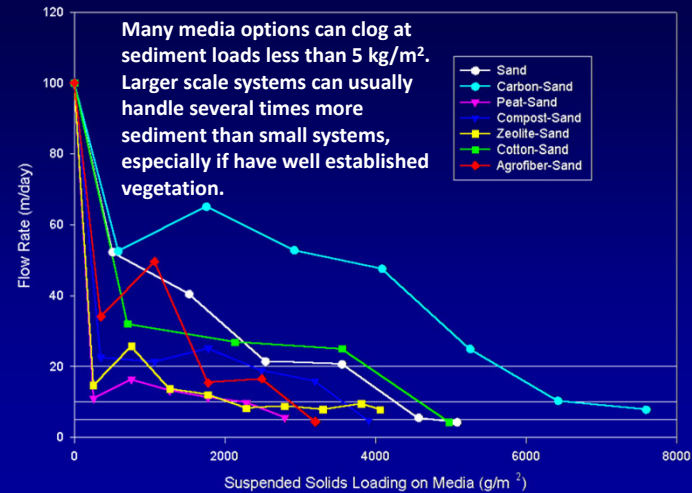


Maintenance with scraping of the surface of the media was not very effective; the removal of several inches of media worked better, but still only for a limited time.

1. Site sand clogged first and had the lowest flow rate
2. Site zeolite and peat alone were next to clog
3. Biofiltration mixed media combination performed better than current site layered media combination

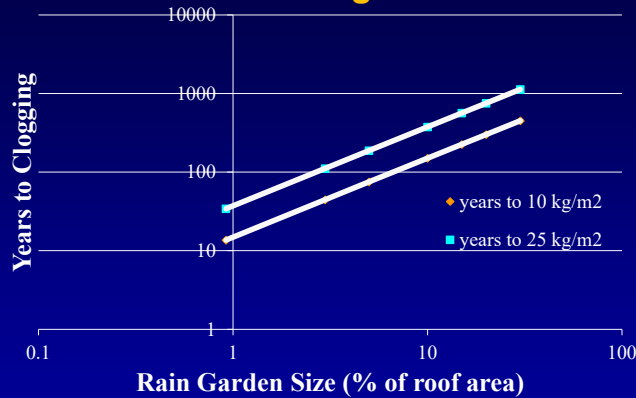
9

### Flow Rate vs. Suspended Solids Loading



10

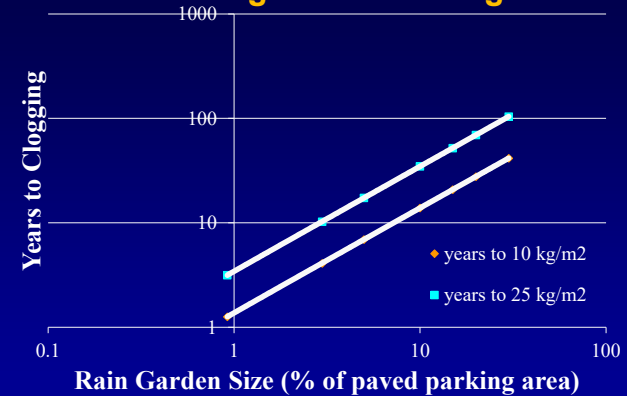
### Clogging Potential for Different Sized Rain Gardens Receiving Roof Runoff



Clogging not likely a problem with rain gardens from roofs

11

### Clogging Potential for Different Sized Rain Gardens Receiving Paved Parking Area Runoff



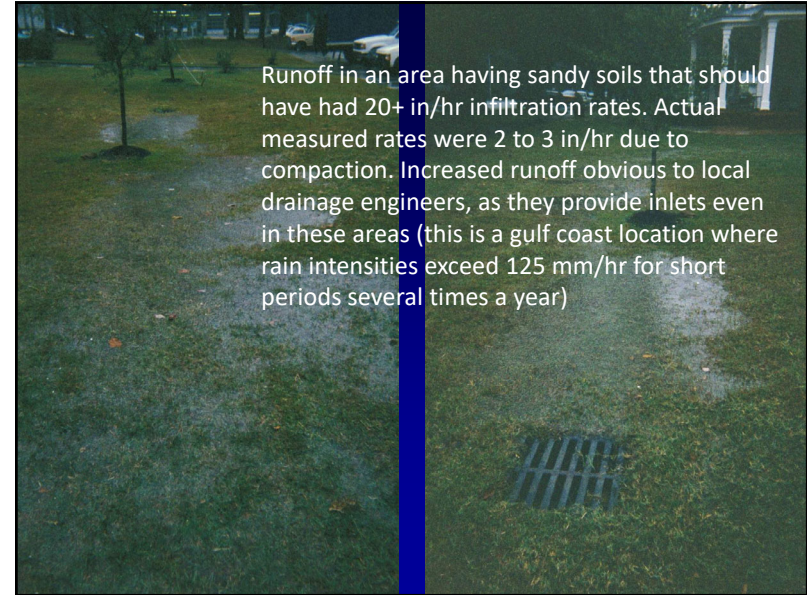
Rain gardens should be at least 10% of the paved drainage area, or receive significant pre-treatment (such as with long grass filters or swales, or media filters) to prevent premature clogging.

12

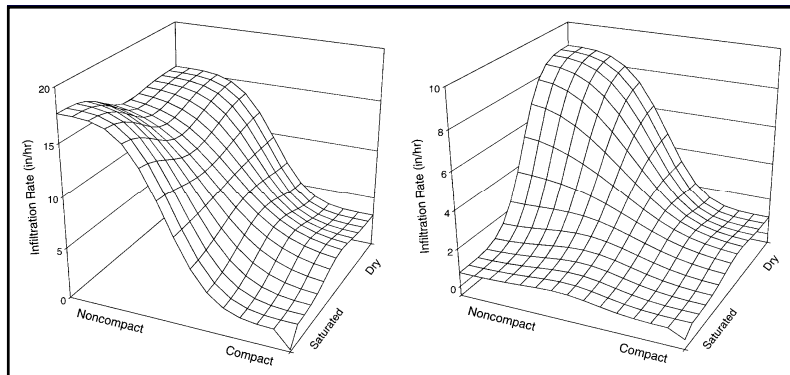
### 3) Compaction

- Compaction of soils or media in an infiltration area (let alone in all pervious areas!) severely hinders infiltration capacity.
- Difficult to recover from compaction, so care is needed during construction and use.

13



14



**Infiltration Rates in Disturbed Sandy Urban Soils**

**Infiltration Rates in Disturbed Clayey Urban Soils**

Pitt, *et al.* 1999

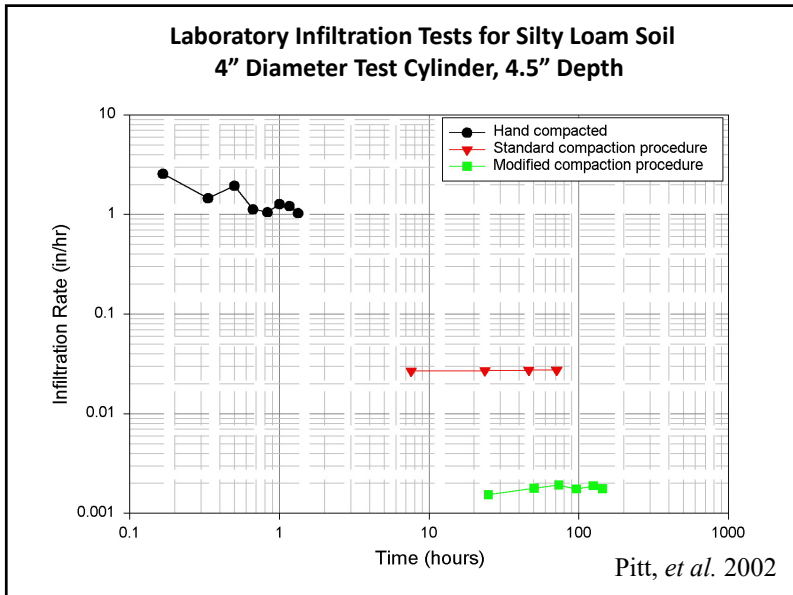
15



In-situ soil density sampling and measurements:

- 1) Small hole is excavated and soil brought to lab for moisture and dry weight analyses (and usually texture measurements also)
- 2) The hole is backfilled with a free-flowing sand to measure the volume
- 3) The soil density is then directly calculated (infiltration rates are also simultaneously measured in the same area)

16



17

## 4) Breakthrough of Chemical Capacity

- Besides sediment clogging, media can fail due to exceeding chemical treatment capacity of the media.
- Long-term column tests more reliable indicators of chemical capacity than short-term batch tests.
- Need to use actual stormwater to represent the wide range of competing chemicals in the water, compared to tests using artificially high concentrations of single pollutant.

18

Mostly ionic forms in filtered stormwater (with some notable exceptions); also, several removal processes occur, beyond ion exchange and sorption

| Analyte   | % Ionic | % Colloidal |
|-----------|---------|-------------|
| Magnesium | 100     | 0           |
| Calcium   | 99.1    | 0.9         |
| Zinc      | 98.7    | 1.3         |
| Iron      | 97      | 3           |
| Chromium  | 94.5    | 5.5         |
| Potassium | 86.7    | 13.3        |
| Lead      | 78.4    | 21.6        |
| Copper    | 77.4    | 22.6        |
| Cadmium   | 10      | 90          |

Morquecho 2005

19

### Breakthrough Capacity Compared to Clogging Period

| Ratios of Media Capacity to Clogging Period | R-SMZ | R-SMZ-GAC | R-SMZ-GAC-PM | Site Sand-GAC-Site Zeolite Layered |
|---|-------|-----------|--------------|------------------------------------|
| Cadmium, Total                              | >230  | >170      | >130         | >150                               |
| Copper, Total                               | >2.2  | >3.4      | >1.7         | >2.2                               |
| Gross Alpha radioactivity                   | >0.3  | >0.3      | >0.2         | >0.2                               |
| Lead, Total                                 | >2.1  | >1.6      | >0.9         | >0.9                               |
| Mercury                                     | >250  | >230      | >130         | >140                               |
| Oil and Grease                              | 0.1   | >0.1      | >0.1         | <0.1                               |
| TCDD  | >3.1  | >2.5      | >1.3         | >1.5                               |

Green: will clog before breakthrough (desired, as easier to detect)  
Red: breakthrough before clogging

20

## 5) Sodium Adsorption Ratio (SAR)

- Excessive amounts of sodium in relation to calcium and magnesium causes the dispersion of clays in a soil, severely restricting infiltration capacity.
- Problem when deicing salts and snowmelt entering infiltration devices that have even small amounts of clay in the soil or media mixture.
- Not much of an issue for roof runoff rain gardens (as long as heavily salted walks or driveways do not drain towards them).
- Acceptable media and soil mixtures should prohibit clays, focusing on sandy material with stable organic amendments (peat recommended; compost can be a problem).

21

A new infiltration pond after first winter; receives snowmelt from adjacent salted parking areas (plus sediment from area construction); lost almost all of the infiltration capacity and is rapidly becoming a (poorly designed) wet pond.



22

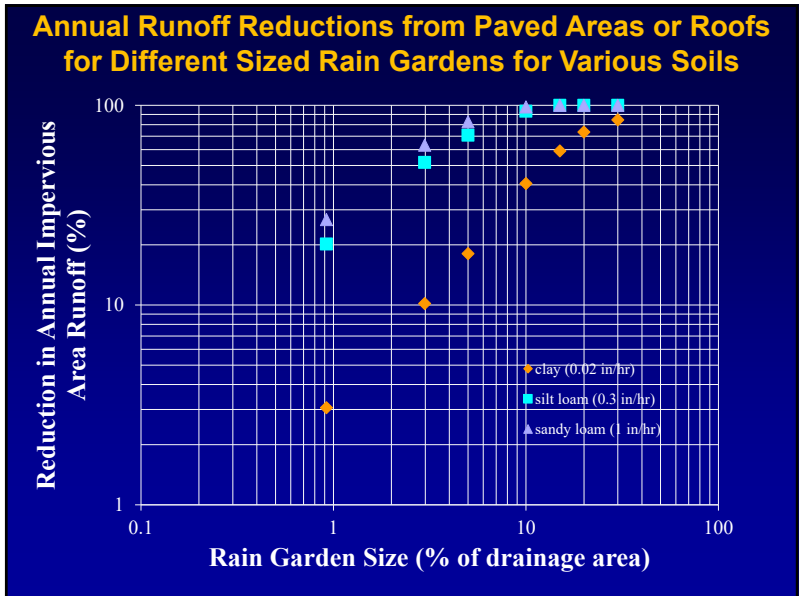


23

## 6) Improper Sizing and Poor Locations

- Improper modeling and design storm use can result in (usually) overly-optimistic performance expectations.
- Long-term simulations needed to assess likely failures and maintenance issues.
- Over-sizing is usually needed (especially in northern climates) to overcome many uncertainties in infiltration behavior.

24



25

### Cistern/Storage Tank Sizing vs. Performance

| Storage per house (ft <sup>3</sup> per ft <sup>2</sup> of roof area) | Reduction in annual roof runoff (%) | Number of 35 gallon rain barrels for 945 ft <sup>2</sup> roof | Tank height size required if 5 ft D (ft) | Tank height size required if 10 ft D (ft) |
|--|-------------------------------------|---|--|---|
| 0.005  | 24                                  | 1   | 0.24                                     | 0.060                                     |
| 0.010  | 29                                  | 2   | 0.45                                     | 0.12                                      |
| 0.020  | 39                                  | 4   | 0.96                                     | 0.24                                      |
| 0.050  | 56                                  | 10  | 2.4                                      | 0.60                                      |
| 0.12   | 74                                  | 25  | 6.0                                      | 1.5                                       |
| 0.50   | 99                                  | 100   | 24                                       | 6.0                                       |

26



27

## 7) Groundwater Interactions

- Groundwater contamination potential from infiltrating stormwater is decreased with treatment before discharge to the groundwater, proper media selection, or located in an area having little contamination potential.
- Mounding below infiltration sites can severely reduce infiltration rates
- Increased groundwater recharge may increase groundwater flows to adjacent urban streams (usually a positive outcome, but if groundwater is contaminated, then this is a potential problem).

28

Potential Problem Pollutants were Identified by Pitt, *et al.* (1994 and 1996) Based on a Weak-Link Model Having the Following Components:

- Their **abundance** in stormwater,
- Their **mobility** through the unsaturated zone above the groundwater, and
- Their **treatability** before discharge.

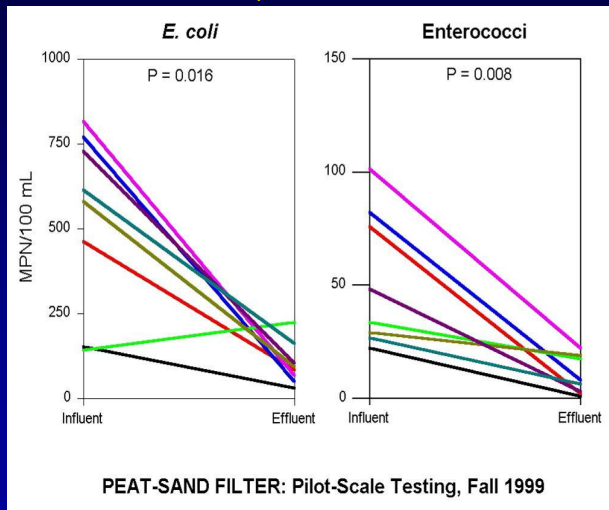
29

### Moderate to High Contamination Potential

| Surface Infiltration after Sedimentation plus sorption/ion-exchange (MCTT and bioretention) | Surface Infiltration with minimal Pretreatment (biofiltration and marginal soils)                         | Injection after Minimal Pretreatment (dry wells, gravel trenches, and most porous pavements)  |
|---|---|---|
|   | Lindane, chlordane  | Lindane, chlordane  |
| Fluoranthene, pyrene  | Benzo (a) anthracene, bis (2-ethylhexyl phthalate), fluoranthene, pentachlorophenol, phenanthrene, pyrene | <b>1,3-dichlorobenzene</b> , benzo (a) anthracene, bis (2-ethylhexyl phthalate), <b>fluoranthene</b> , pentachlorophenol, phenanthrene, <b>pyrene</b> |
| <b>Enteroviruses</b>  | <b>Enteroviruses</b>  | <b>Enteroviruses, some bacteria and protozoa</b>  |
|   |   | <b>Nickel</b> , chromium, lead, <b>zinc</b>   |
| <b>Chloride</b>   | <b>Chloride</b>   | <b>Chloride</b>   |

30

Media treatment can be quite effective for bacteria reductions

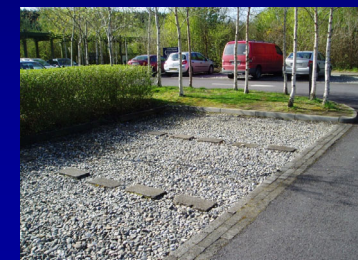
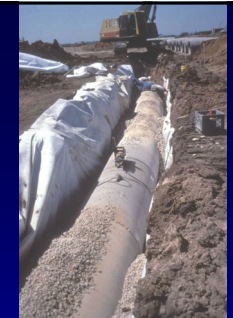


Clark and Pitt 1999

31

Minimal Pre-treatment before Infiltration Leads to Greater Groundwater Contamination Potential

Early approaches to infiltrating stormwater:



(also, filter fabric liners are usually not recommended anymore as many have failed due to clogging from silts)

32



## 8) Improper Construction and Poor Selection of Components

- Problems with media materials (did I mention clay before?)
- Over-filling biofilters (a surprisingly common problem), reducing storage capacity, usually with overflows set at too low of an elevation further decreasing storage.
- Difficult for water to enter device (not in flow path, no gradient, blocked entrances, and no drop off to top of media, allowing build-up of debris).

33



Red southern clay turned these terraced biofilters into something else. Constant flooding killed the vegetation and they are attempting to break up the surface clay layers.

34



Over-filled biofilter allowing short-circuiting of surface flows to slot drain inlet that is set slightly below top of media

35



Obvious short-circuiting flow path and washout of media in over-filled biofilter.

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## 9) Poor Maintenance

- Proper maintenance is necessary to ensure expected performance.
- Excessive erosion of surrounding areas and at the device itself can lead to excessive sediment loads and clogging in a short period of time.
- Irrigation is needed during periods of low rainfall to keep biofilter plants alive and active. Similar needs for green roof plants during seasonal dry periods. Plants need to be selected to withstand a wide range of dry to flooding conditions.

37

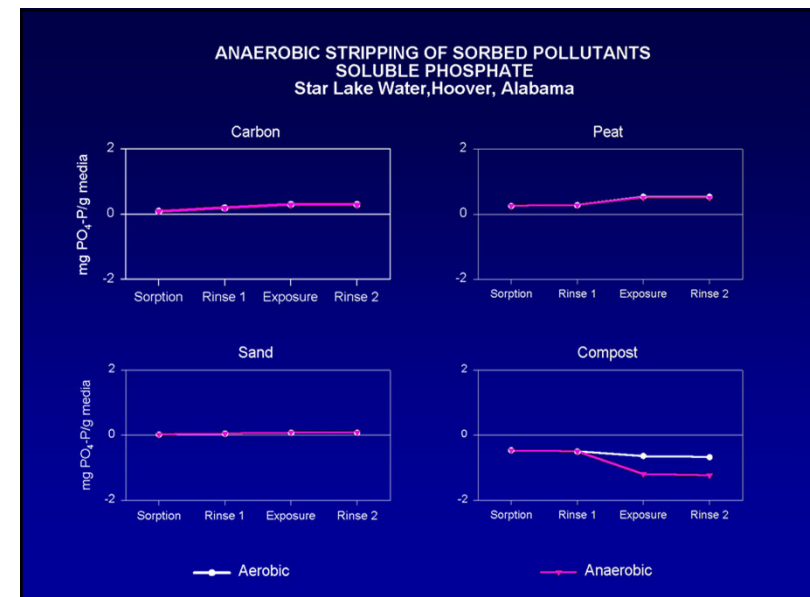


38

## 10) Anaerobic Conditions

- Anaerobic conditions in biofilter media can enhance nitrate removals, if used in conjunction with other properly designed attributes (media selection and underdrain design).
- Many organic media can lose previously captured pollutants, especially nutrients, under anaerobic conditions. Metal retention is usually more secure, but degradation of the media results in losses of all materials.
- “First-flushes” of retained water from biofilters that have gone anaerobic contain very high pollutant concentrations.
- Free-draining media that remain aerobic during interevent periods exhibit fewer of these problems.

39



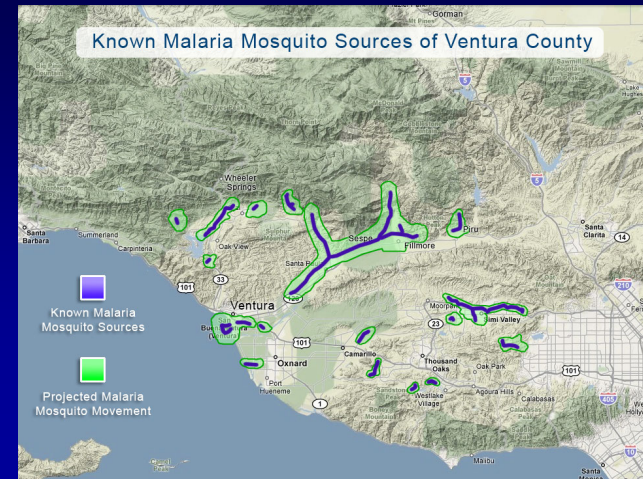
40

## 11) Large Underdrains and Short-Circuiting

- Underdrains are needed in areas where standing water for extended periods of time causes problems, and to reduce anaerobic conditions in biofilter media.
- Conventional large underdrains provide too large of a drainage flow rate causing short-circuiting and short residence times.
- Flow restrictions are causes of clogging or maintenance problems.
- Modified underdrains can provide a good solution.

41

## Many Areas Require Biofilter Drainage within 72 hours to Prevent Mosquito Infestation



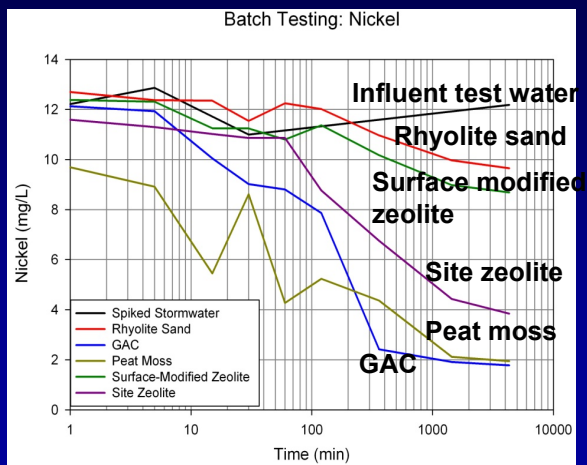
42

## Contact Time Affects Pollutant Removals

Minimal filtered metal removal observed for all media except peat when contact time <10 minutes.

The optimal contact times for filtered metals removal ranged from 10 to 1,000 minutes (17 hrs), depending on the metal and the media type.

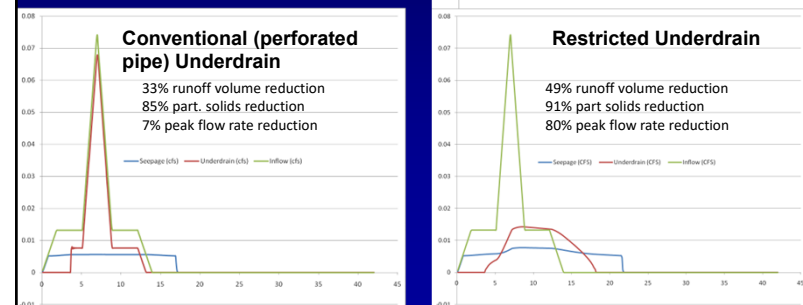
However too long of a contact time increased leaching losses from some media.



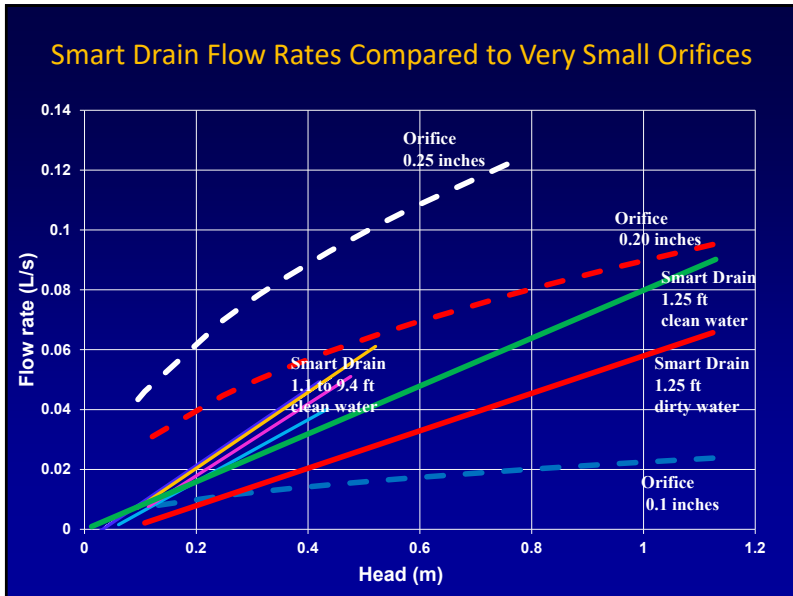
43

## Underdrain Effects on Water Balance

20 mm rain with complex inflow hydrograph from 0.4 ha of pavement. 2.2% of paved area is biofilter surface, with natural loam soil (12 mm/hr infiltr. rate) and 0.6 m of modified fill soil for water treatment and to protect groundwater.



44

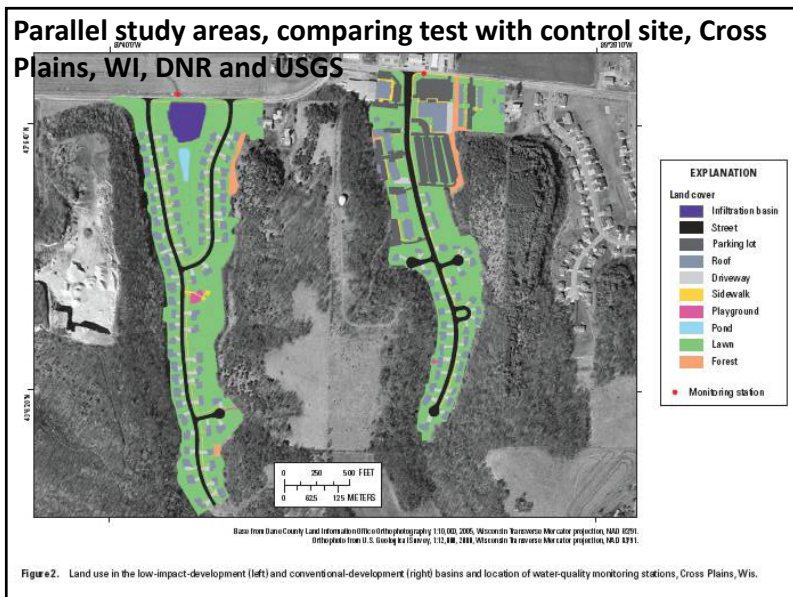


45

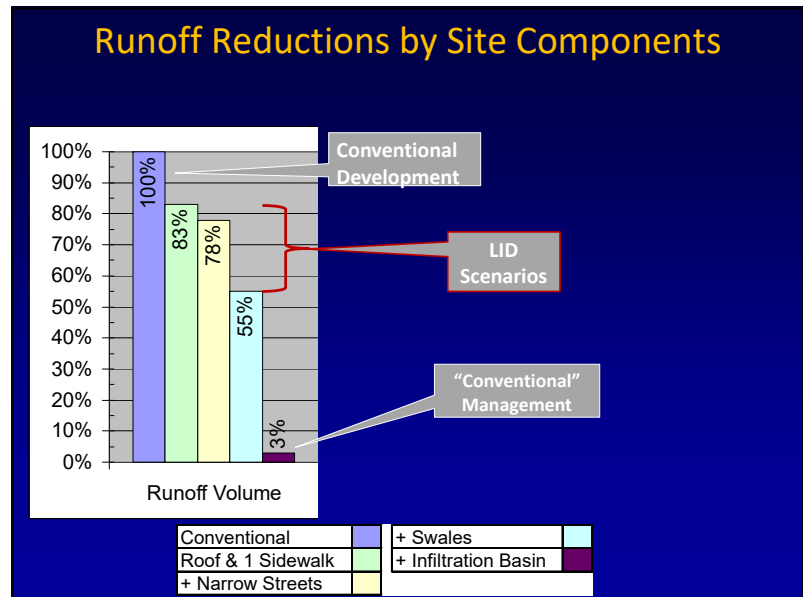
## 12) Need Combinations of Controls (storage, sedimentation and infiltration)

- Infiltration alone can be effective in reducing most stormwater pollutants and flows.
- Sedimentation before infiltration offers advantages of pre-treatment and better sediment control.
- Storage before infiltration enhances treatment at low treatment flow rates.

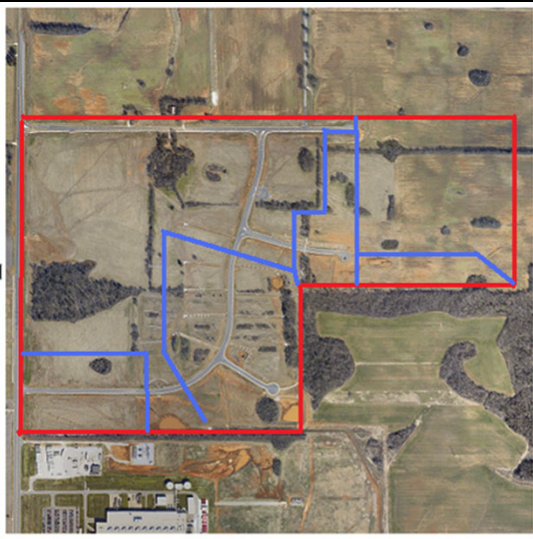
46



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48

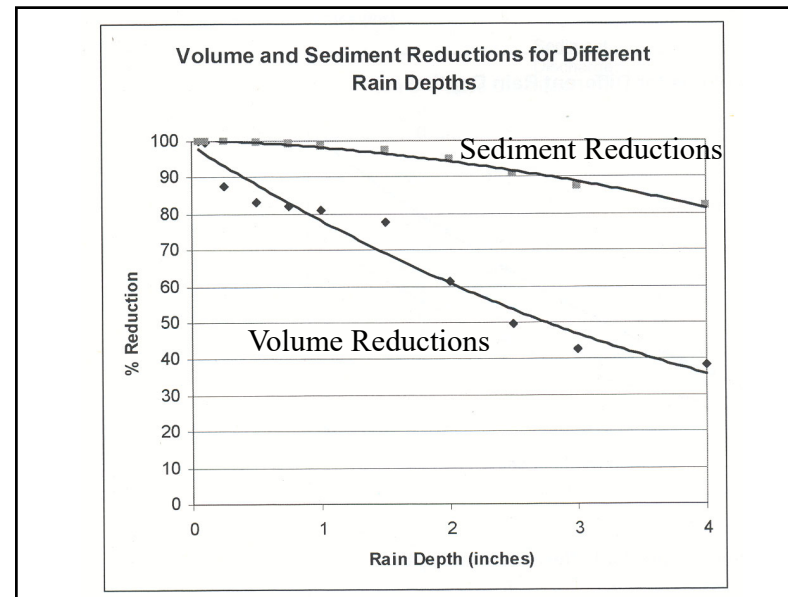


**North Huntsville (AL) Conservation Design Industrial Park**

- On-site bioretention swales
- Level spreaders
- Large regional swales
- Wet detention ponds
- Critical source area controls
- Pollution prevention (no Zn!)
- Buffers around sinkholes
- Extensive trail system linking water features and open space

Aerial Photo of Site under Construction (Google Earth)

49




50

**Modeling Green Infrastructure Compared with Large-Scale Monitoring at Kansas City, MO and Cincinnati, OH**

51

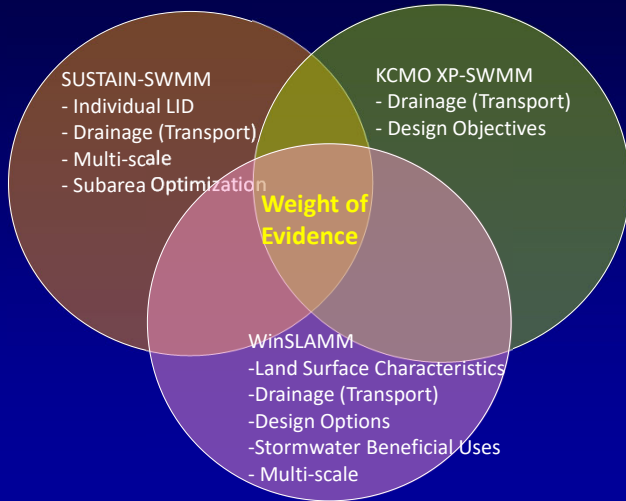
**Kansas City's CSO Challenge**

- Combined sewer area: 58 mi<sup>2</sup>
- Fully developed
- Rainfall: 37 in./yr
- 36 sewer overflows/yr by rain > 0.6 in; reduce frequency by 65%.
- 6.4 billion gal overflow/yr, reduce to 1.4 billion gal/yr
- Aging wastewater infrastructure
- Sewer backups
- Poor receiving-water quality



52

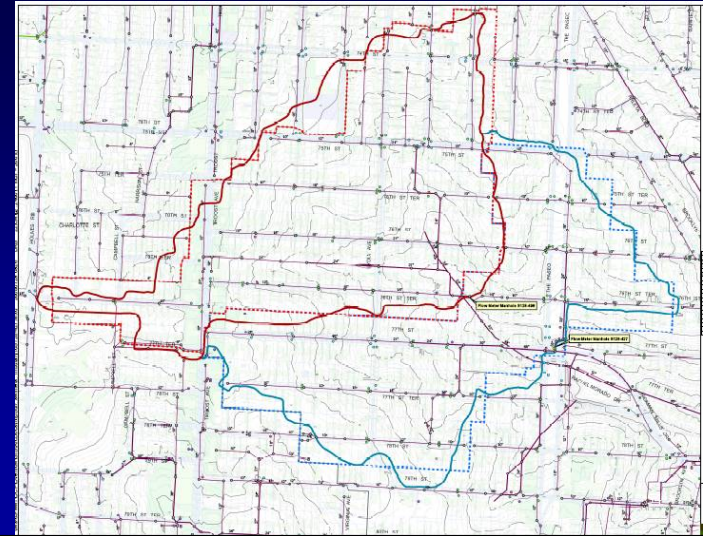
## KC's Modeling Connections



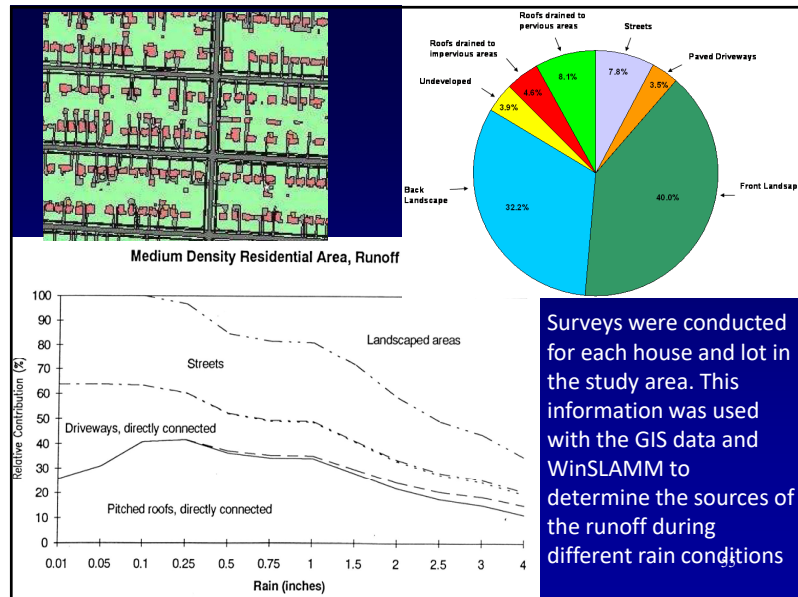
53

53

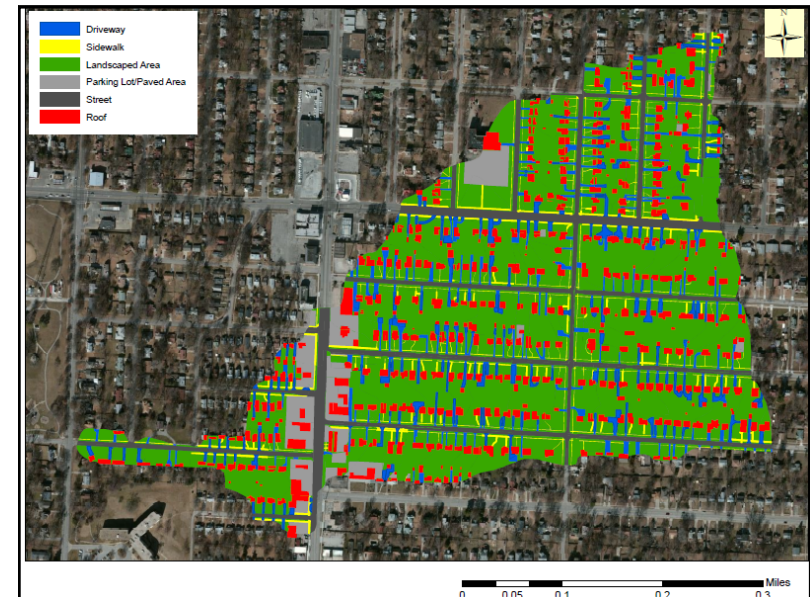
## Adjacent Test and Control Watersheds



54



55



56

## Major Land Use Components in Residential Portion of Study Area (% of area and % of total annual flow contributions)

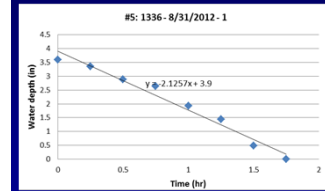
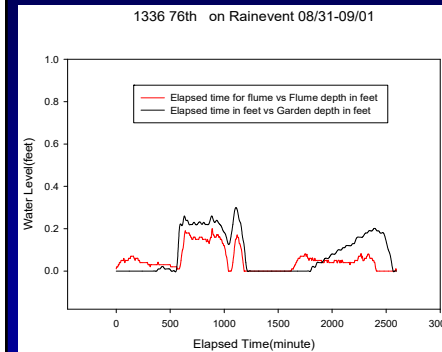
|                    | Roofs   | Drive-ways | Side-walks | Park-ing | Streets | Land-scaped | Total   |
|--------------------|---------|------------|------------|----------|---------|-------------|---------|
| Directly connected | 2 (6)   | 4 (9)      | 1 (3)      | 2 (5)    | 9 (21)  |             | 18 (44) |
| Disconnected       | 11 (7)  | 4 (3)      | 1 (1)      |          |         |             | 16 (11) |
| Landscaped         |         |            |            |          |         | 66 (45)     | 66 (45) |
| Total area         | 13 (13) | 8 (12)     | 2 (4)      | 2 (5)    | 9 (21)  | 66 (45)     | 100     |

Based on KCMO GIS mapping and detailed site surveys, along with WinSLAMM calculations.

57

57

## Example Water Level in Influent Flume and Water Stage Recordings in Biofilter used for Calculating Infiltration Rates during Rains

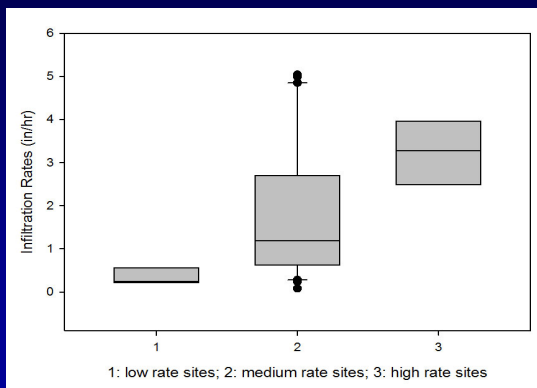


Example plot of recession limbs and infiltration rate calculation (after influent flows ceased).

58

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## Measured Biofilter Infiltration Rates During Actual Rains, Separated into Three Categories

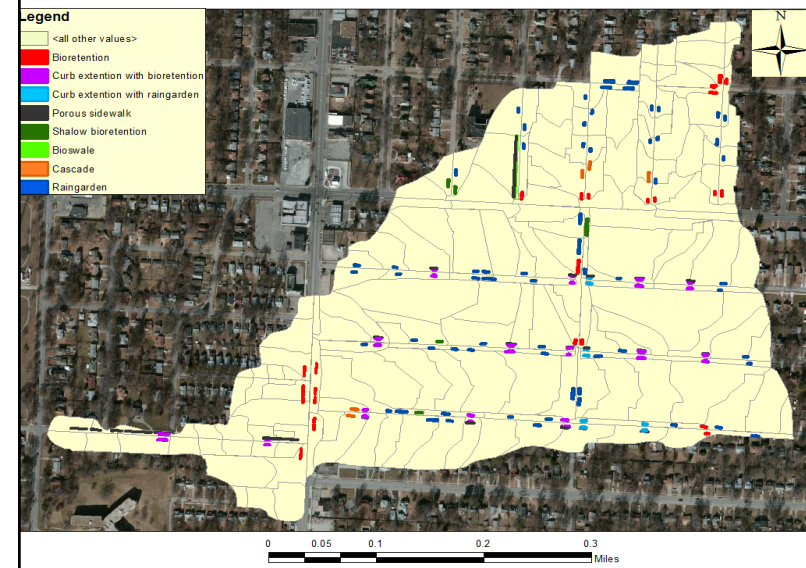


1: low rate sites; 2: medium rate sites; 3: high rate sites

59

59

## 100-acre Pilot Study Area

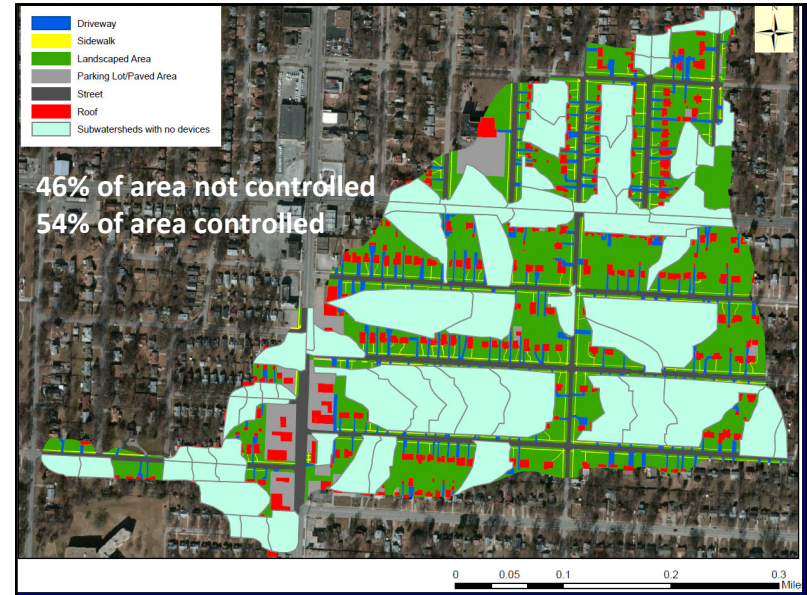


60

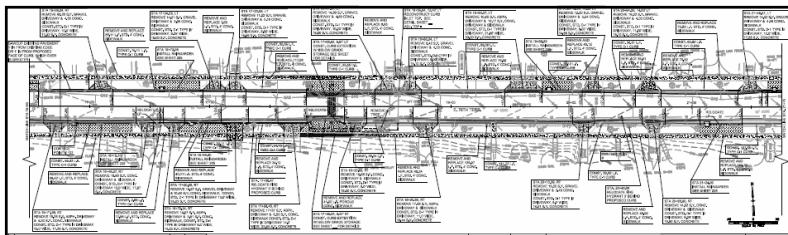
### Summary of Constructed Stormwater Controls in Test Area

| Design plan component       | Number of this type of stormwater control units in 100 acre test (pilot) area | Device as a % of the drainage area | Average drainage area for each unit (ac) | Total area treated by these devices (ac) |
|-----------------------------|---|------------------------------------|--|--|
| Bioretention                | 24 (no curb extensions)   | 1.6                                | 0.40                                     | 9.6                                      |
|                             | 28 (with curb extension)  | 1.5                                | 0.40                                     | 11.2                                     |
|                             | 5 (shallow)   | 1.6                                | 0.40                                     | 2.0                                      |
| Bioswale                    | 1 (vegetated swale)   | 8.9                                | 0.50                                     | 0.5                                      |
| Cascade                     | 5 (terraced bioretention cells in series)                                     | 1.9                                | 0.40                                     | 2.0                                      |
|                             | 18 (with underdrains)   | 100.0                              | 0.015                                    | 0.3                                      |
| Porous sidewalk or pavement | 5 (with underground storage cubes)  | 99.9                               | 0.015                                    | 0.1                                      |
|                             | 64 (no curb extensions)   | 2.8                                | 0.40                                     | 25.6                                     |
| Rain garden                 | 8 (with curb extension)   | 1.5                                | 0.40                                     | 3.2                                      |

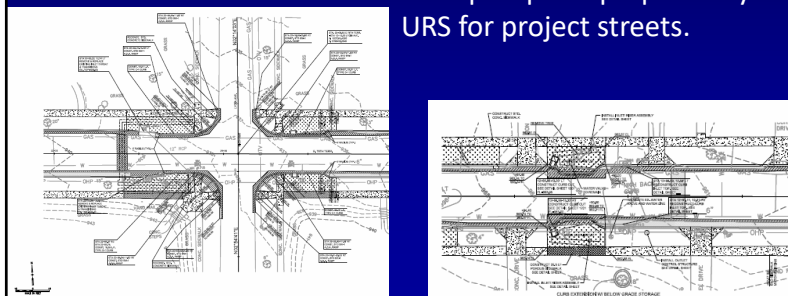
61



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Examples plans prepared by URS for project streets.



63



64

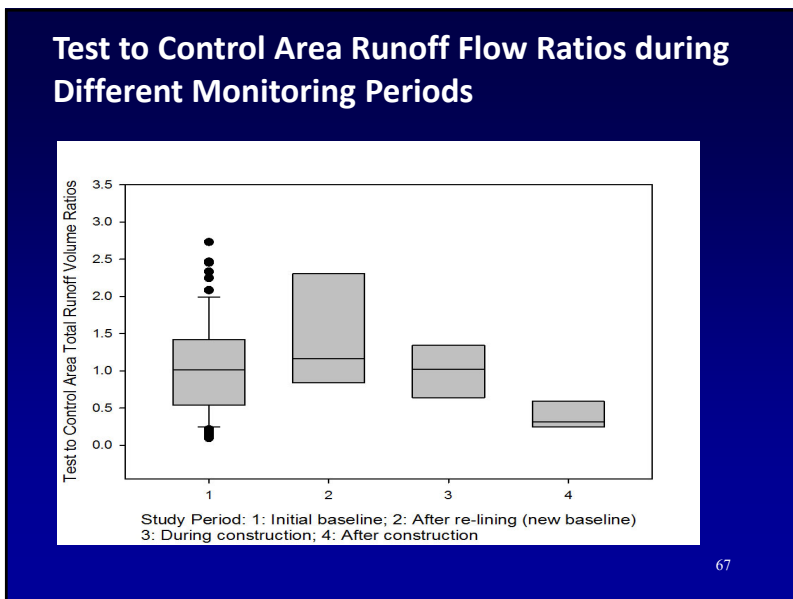




65



66



67

### Test and Control Watershed Flow Comparisons

| Monitoring Period                        | % change compared to initial baseline (and p from Wilcoxon Rank-Sum test) |
|--|---|
| Initial baseline                         | n/a   |
| After re-lining                          | 44% increase (p=0.20)   |
| During construction                      | 4% decrease (p=0.94)  |
| After construction (after April 1, 2012) | 55% decrease (p=0.006)  |

68

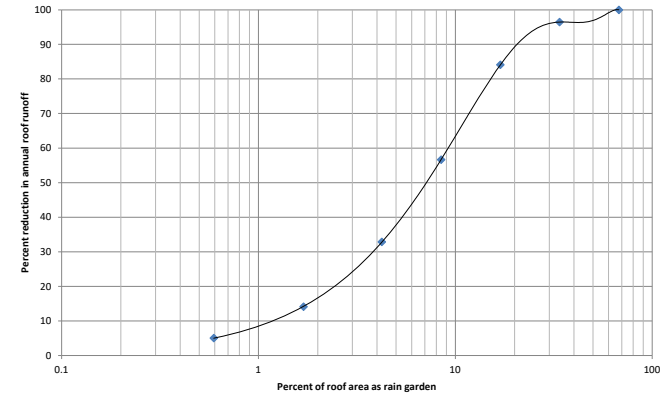
68

One of the Kansas City rain gardens being monitored (zero surface discharges during the three years of monitoring; this rain garden is 20% of roof drainage area)



69

### Percentage Reductions of Annual Runoff Flows with Rain Gardens



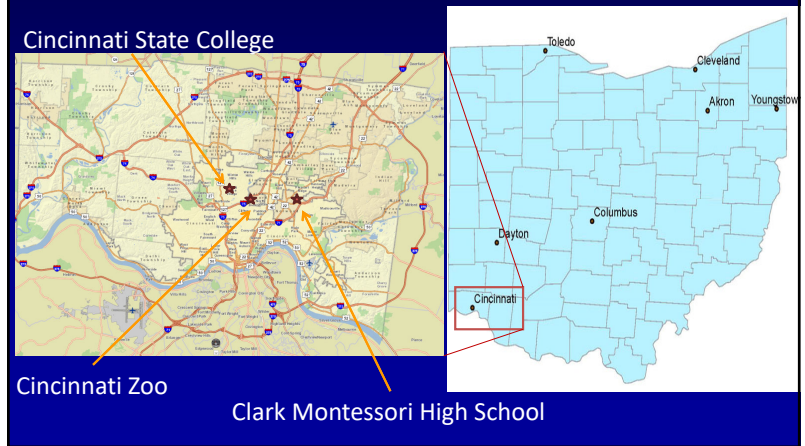
70

### Cincinnati Demonstration Projects Background

- The Metropolitan Sewer District of Greater Cincinnati (MSD) is among the top 5 Combined Sewer Overflow (CSO) dischargers in the country, discharging approximately 14 billion gallons of overflows during a typical year of rainfall.
- About 75 million gallons per year of stormwater removed from the combined system from 22 Green Demonstration projects which include:
  - 290,000 square feet of bioinfiltration practices,
  - 168,000 square feet of vegetative (green) roofs,
  - 155,000 square feet of porous/pervious pavement,
  - 125,000 gallons of rainwater storage for reuse,
  - 2,040 linear feet of storm sewer separation, and
  - 5 large capacity stormwater dry wells.

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### Geographical Locations and Description – Cincinnati, OH



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## Availability of Flow Monitoring Data for Different Green Infrastructure Evaluation Locations

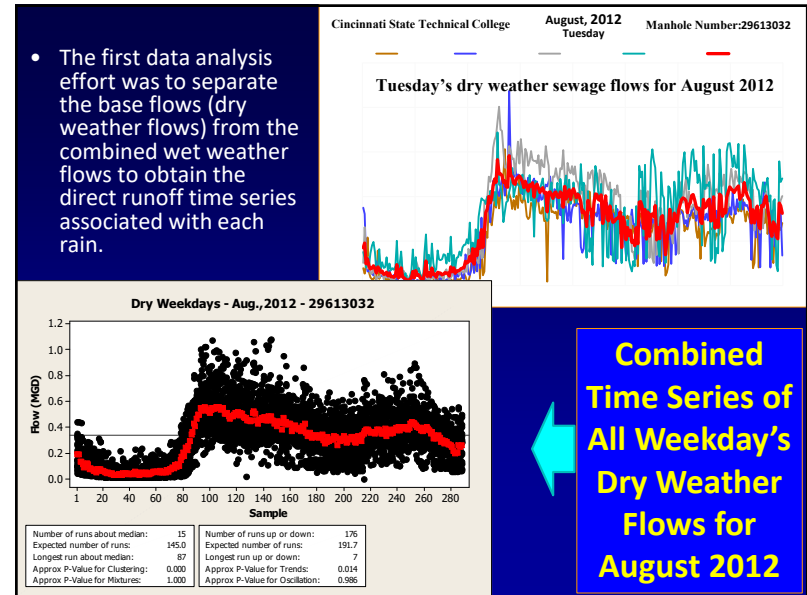
About 3 years of high-resolution (5-minute) flow measurements from in-system flow monitors located in combined and separate sewers on or adjacent to several green infrastructure installations

| Location  | Jan-10 | Feb-10 | Mar-10 | Apr-10 | May-10 | Jun-10 | Jul-10 | Aug-10 | Sep-10 | Oct-10 | Nov-10 | Dec-10 | Jan-11 | Feb-11 | Mar-11 | Apr-11 | May-11 | Jun-11 | Jul-11 | Aug-11 | Sep-11 | Oct-11 | Nov-11 | Dec-11 |
|---|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|
| Cincinnati State College Combined Sewer (above & below site monitoring) |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |
| Cincinnati State College Separate Sewer (single monitoring location)    |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |
| Cincinnati Zoo - Main Entrance (separate sewer)                         |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |
| Cincinnati Zoo - African Savannah (combined sewer)                      |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |
| Clark Montessori High School (combined sewer)                           |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |        |

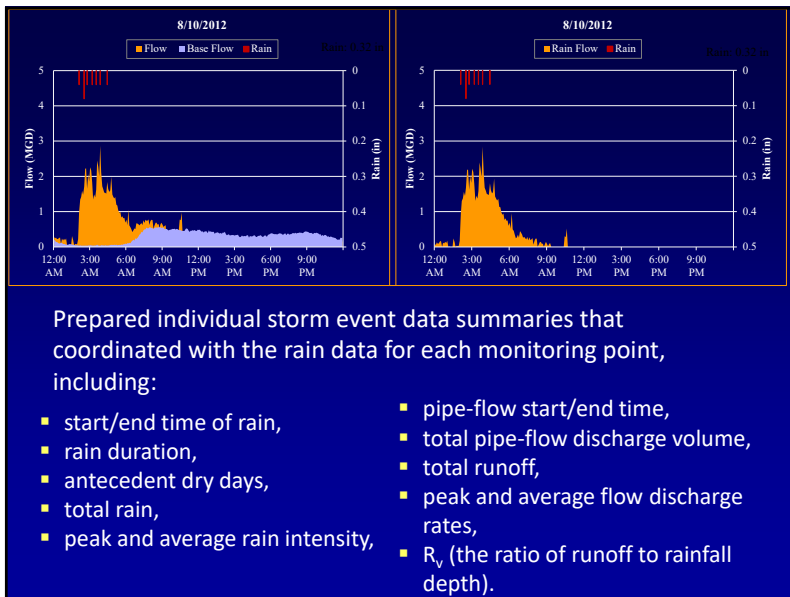
  

|  |
|--|
| <span style="background-color: #f8d7da; border: 1px solid #c6c8ca; padding: 2px;"> </span> Before Construction |
| <span style="background-color: #fff3cd; border: 1px solid #ffee58; padding: 2px;"> </span> During Construction |
| <span style="background-color: #d4edda; border: 1px solid #c3e6cb; padding: 2px;"> </span> After Construction  |

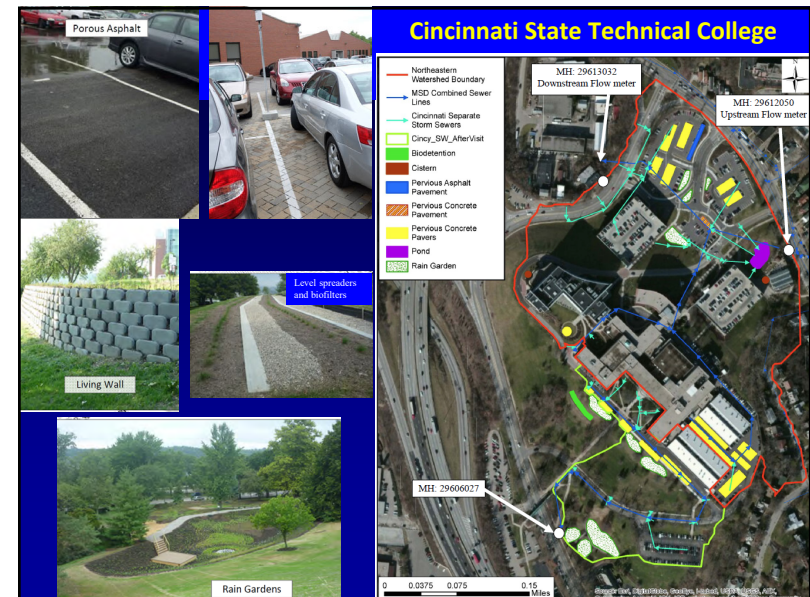
73



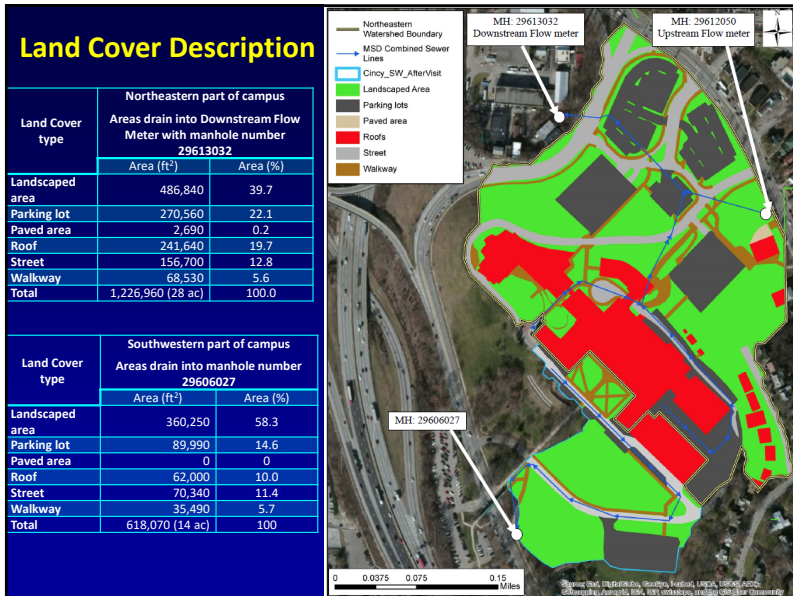
74



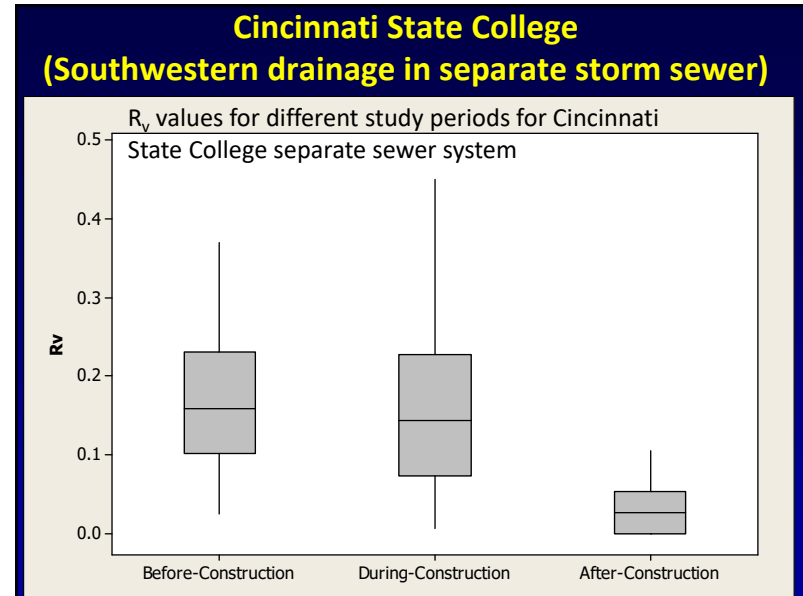
75



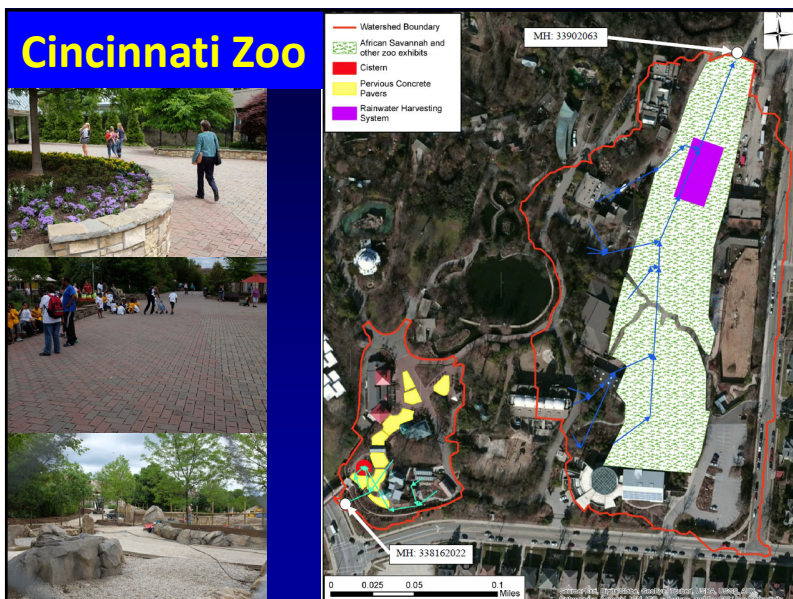
76



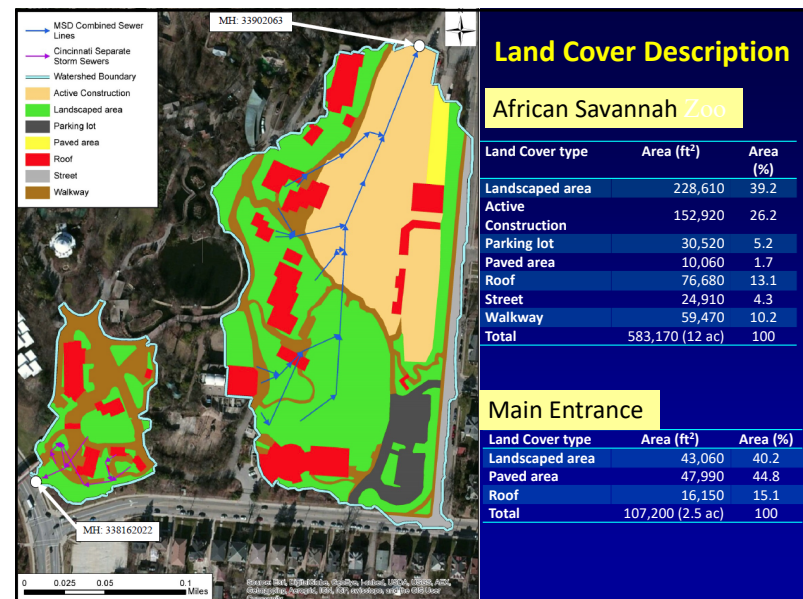
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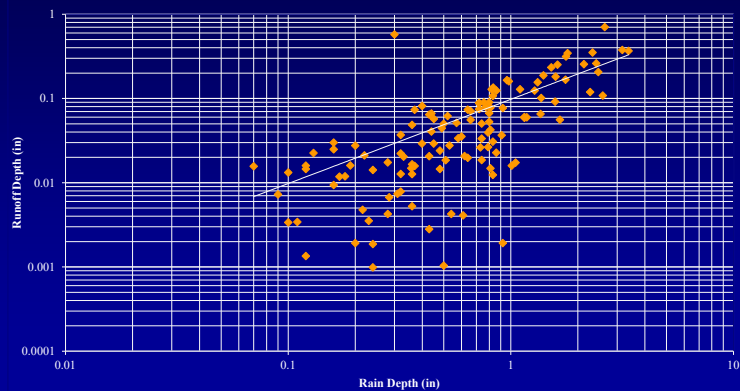


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## Cincinnati Zoo – Main Entrance (Paver blocks)

176 after construction events

Average  $R_v = 0.10$  based on observed slope



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## Clark Montessori High School



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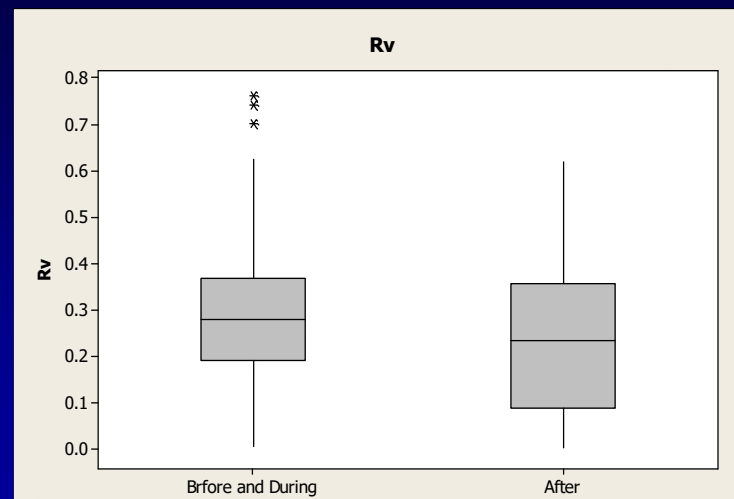
## Land Cover Description

| Land Cover type | Area (ft <sup>2</sup> ) | Area (%)     |
|-----------------|-------------------------|--------------|
| Driveway        | 22,840                  | 3.6          |
| Landscaped area | 369,450                 | 57.5         |
| Parking lot     | 22,080                  | 3.4          |
| Paved area      | 15,030                  | 2.3          |
| Roof            | 86,620                  | 13.5         |
| Soccer Field    | 25,870                  | 4.0          |
| Street          | 86,130                  | 13.4         |
| Walkway         | 14,960                  | 2.3          |
| <b>Total</b>    | <b>642,980 (15 ac)</b>  | <b>100.0</b> |



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## Clark Montessori High School



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## Summary

| Location  | Runoff Volume Reduction (%) Compared to Pre-Construction Data                                       |
|---|---|
| Cincinnati State College – Southern Area (bioinfiltration and rain gardens)                         | 80  |
| Cincinnati Zoo – Main Entrance (extensive paver blocks)   | Average Rv values after construction: 0.1 (compared to about 0.8 for conventional pavement in area) |
| Cincinnati Zoo – African Savannah (rainwater harvesting system and pavement removal)                | 70  |
| Clark Montessori High School (green roofs and parking lot biofilters on small portion of watershed) | 21  |

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## Conclusions and Recommendations for Flow Monitoring for Green Infrastructure Performance

- Monitor both test and control areas both before and after construction of stormwater controls, if possible, for the greatest reliability (to account for typical year-to-year rainfall variations and to detect sensor problems early).
- Test areas should have most of their flows treated by the control practices to maximize measurable reductions.
  - **Any untreated upgradient areas should be very small in comparison to the test areas. Difficult to subtract two large numbers (each having measurement errors and other sources of variability), such as above and down gradient monitoring stations, and have confidence on the targeted flows.**

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## Summary

- Learn from others (and yourselves)! Evaluate and monitor installations and modify approaches.
- Site conditions and local rains dramatically affect performance.
- Northern areas and locations using deicing salts are an extreme example that require special approaches to stormwater management.
- Groundwater issues need to be considered.
- Combinations of unit processes almost always result in the most robust, most cost-effective, and best water quality.

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