

WinSLAMM v 10.3 Porous Pavement


Robert Pitt, P.E., Ph.D., BCEE
Emeritus Cudworth Professor of Urban Water Systems
University of Alabama

1

1

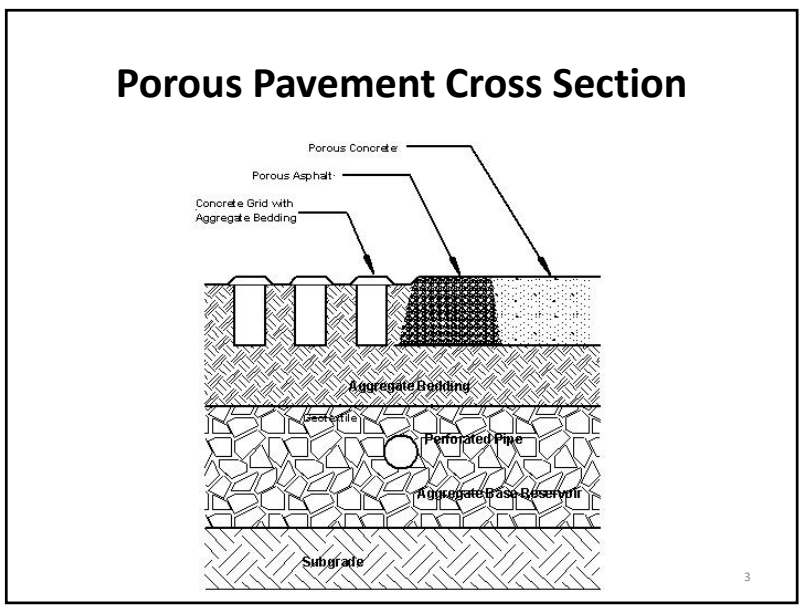
We will cover . . .

- Porous Pavement Options in WinSLAMM
- Porous Pavement Performance Algorithm
- Entering Porous Pavement Data into the Program
- Modeling Notes
- Example Input and Output

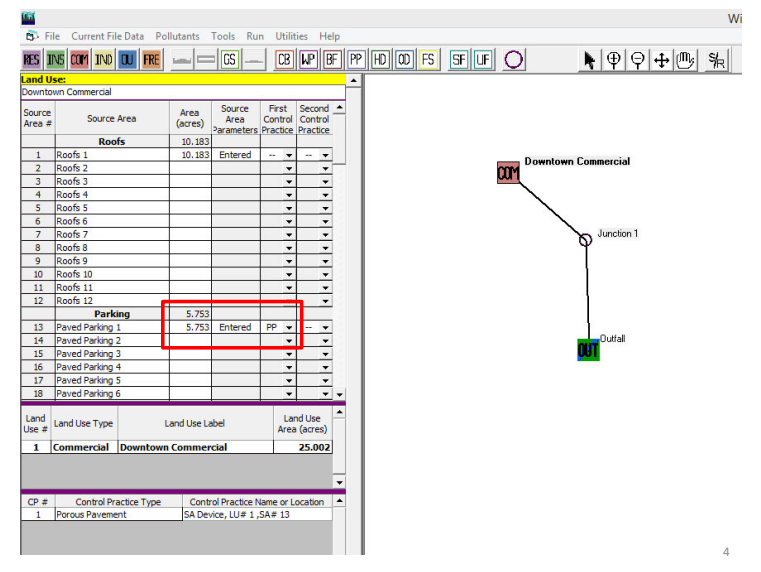


2

2



3



Source Area #	Source Area	Area (acres)	Source Area	First Control Parameters	Second Control Practice
Roofs					
1	Roofs 1	10.183	Entered	---	---
2	Roofs 2				
3	Roofs 3				
4	Roofs 4				
5	Roofs 5				
6	Roofs 6				
7	Roofs 7				
8	Roofs 8				
9	Roofs 9				
10	Roofs 10				
11	Roofs 11				
12	Roofs 12				
Parking					
13	Paved Parking 1	5.753	Entered	PP	---
14	Paved Parking 2	5.753	Entered	PP	---
15	Paved Parking 3				
16	Paved Parking 4				
17	Paved Parking 5				
18	Paved Parking 6				

4

4

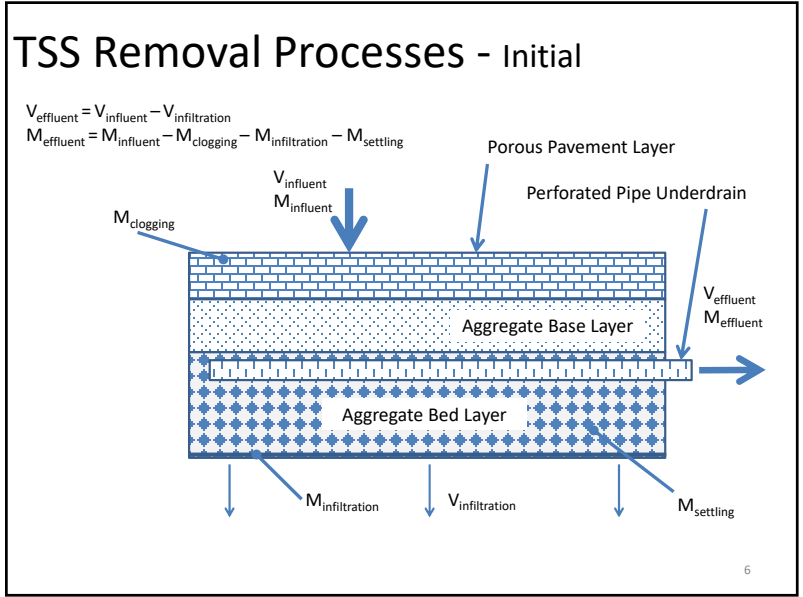
Source Area #	Source Area	Area (acres)	Source Area Parameters	First Control Practice	Second Control Practice
Roofs					
1	Roofs 1	10.183	Entered	--	--
2	Roofs 2				
3	Roofs 3				
4	Roofs 4				
5	Roofs 5				
6	Roofs 6				
7	Roofs 7				
8	Roofs 8				
9	Roofs 9				
10	Roofs 10				
11	Roofs 11				
12	Roofs 12				
Parking					
13	Paved Parking 1	5.753	Entered	--	--
14	Paved Parking 2				
15	Paved Parking 3				
16	Paved Parking 4				

Land Use #	Land Use Type	Land Use Label	Land Use Area (acres)
1	Commercial	Downtown Commercial	15.936

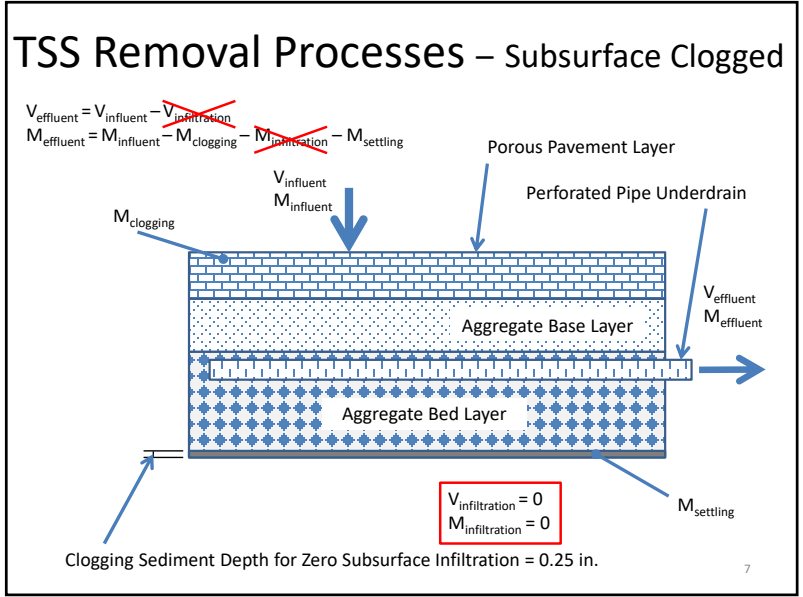
CP #	Control Practice Type	Control Practice Name or Location
1	Porous Pavement	DS Porous Pavement # 1

WinSLAMM version v 10.3 allows both a Source Area and a Drainage System Porous Pavement Control

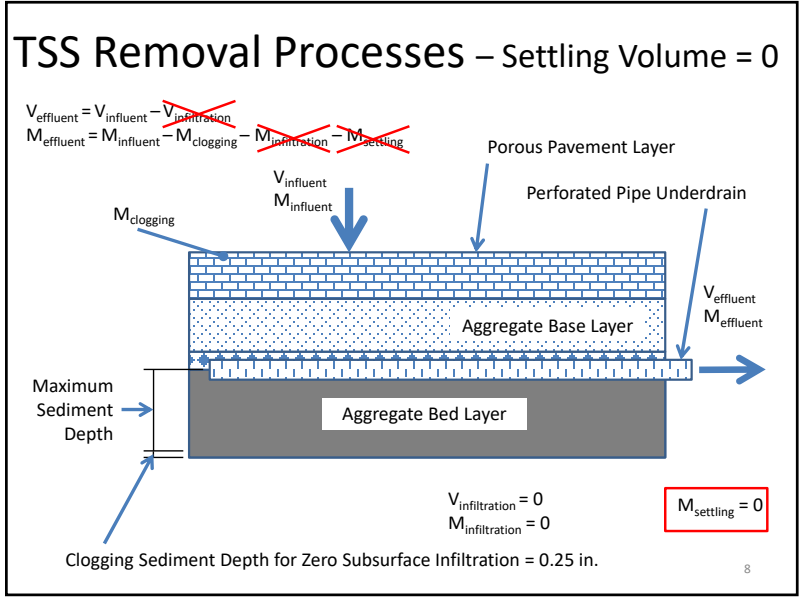
5



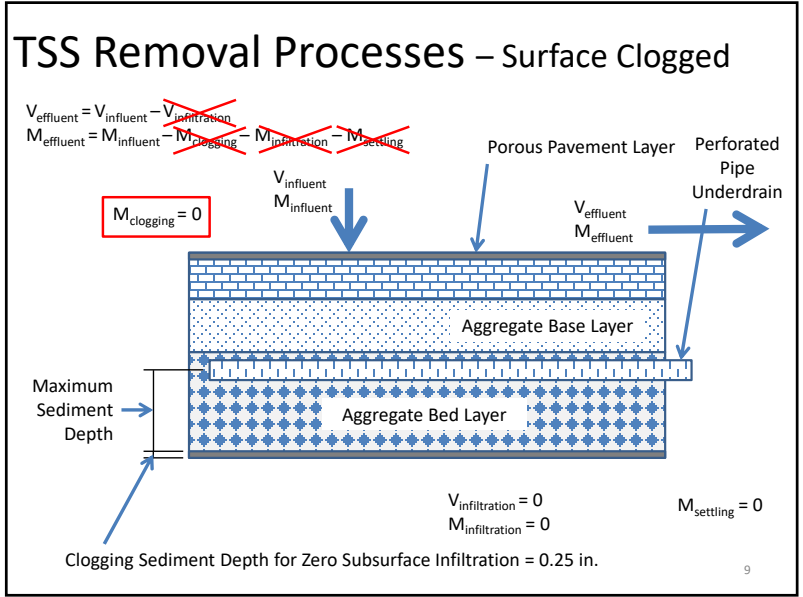
6



7



8



9

Entering Porous Pavement Data into the Program

- Pavement Geometry and Properties
- Outlet/Discharge Options
- Surface Pavement Layer and Cleaning Data
- Native Soil Infiltration Data

10

Area - runoff from the Total Area drains to the Porous pavement area

Pavement Area Graphic

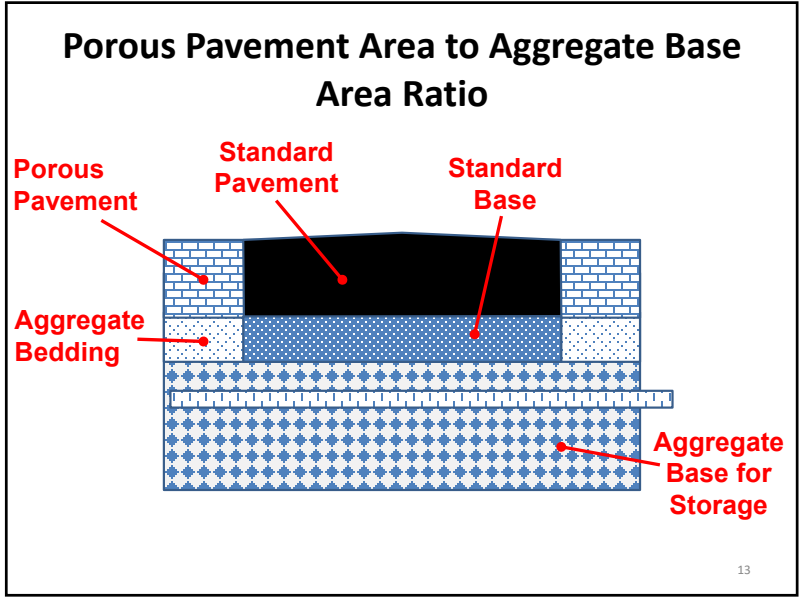
Run-on Allowed

11

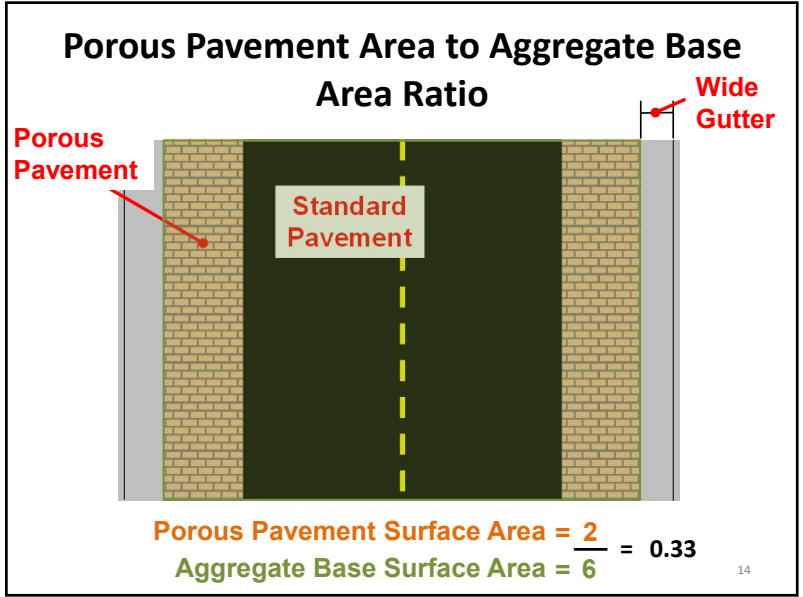
Pavement Geometry and Properties

1 - Pavement Thickness (in)	3.0
Pavement Porosity (>0 and <1)	0.25
2 - Aggregate Bedding Thickness (in)	9.0
Aggregate Bedding Porosity (>0 and <1)	0.25
3 - Aggregate Base Reservoir Thickness (in)	9.0
Aggregate Base Reservoir Porosity (>0 and <1)	0.25
4 - Aggregate Base Reservoir Thickness (in)	9.0
Aggregate Base Reservoir Porosity (>0 and <1)	0.25
Porous Pavement Area to Agg Base Area Ratio	1.00

12



13



14

Poros Pavement Control Device

First Source Area Control Practice
Land Use: Commercial 1
Source Area: Paved Parking 1
Total Porous and Impervious Pavement Area: 1.000 ac.
Porous pavement area (acres): 0.400
Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Surface Pavement Layer Infiltration Rate Data
Initial Infiltration Rate (in/hr): 100.00
Surface Pavement Percent Solids Removal Upon Cleaning (0-100): 50.0

Restorative Cleaning Frequency
 Never Cleaned
 Three Times per Year
 Semi-Annually
 Annually
 Every Two Years
 Every Three Years
 Every Four Years
 Every Five Years
 Every Seven Years
 Every Ten Years

Outlet/Discharge Options
 Perforated Pipe Underdrain Diameter, if used (inches): 3.00
 4 - Perforated Pipe Underdrain Outlet Invert Elevation (inches above Datum): 6.0
 Number of Perforated Pipe Underdrains (<250): 1
 Subgrade Seepage Rate (in/hr) - select below or enter: 0.050
 Use Random Number Generation to Account for Uncertainty in Seepage Rate:
 Subgrade Seepage Rate COV: 1.60
 Underdrain Discharge Percent TSS Reduction (0-100) or leave blank for program to calculate: 0

Select Subgrade Seepage Rate
 Sand - 8 in/hr
 Loamy sand - 2.5 in/hr
 Sandy loam - 1.0 in/hr
 Loam - 0.5 in/hr
 Silt loam - 0.3 in/hr
 Sandy silt loam - 0.2 in/hr
 Clay loam - 0.1 in/hr
 Silty clay loam - 0.05 in/hr
 Sandy clay - 0.05 in/hr
 Silty clay - 0.04 in/hr
 Clay - 0.02 in/hr

Percent of Total Area that is Porous Pavement: 40.0 %

Outlet and Discharge Options

15

Poros Pavement Control Device

First Source Area Control Practice
Land Use: Commercial 1
Source Area: Paved Parking 1
Total Porous and Impervious Pavement Area: 1.000 ac.
Porous pavement area (ac): 0.400
Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Surface Pavement Layer Infiltration Rate Data
Initial Infiltration Rate (in/hr): 100.00
Surface Pavement Percent Solids Removal Upon Cleaning (0-100): 50.0

Restorative Cleaning Frequency
 Never Cleaned
 Three Times per Year
 Semi-Annually
 Annually
 Every Two Years
 Every Three Years
 Every Four Years
 Every Five Years
 Every Seven Years
 Every Ten Years

Outlet/Discharge Options
 Perforated Pipe Underdrain Diameter, if used (inches): 3.00
 4 - Perforated Pipe Underdrain Outlet Invert Elevation (inches above Datum): 6.0
 Number of Perforated Pipe Underdrains (<250): 1
 Subgrade Seepage Rate (in/hr) - select below or enter: 0.050
 Use Random Number Generation to Account for Uncertainty in Seepage Rate:
 Subgrade Seepage Rate COV: 1.60
 Underdrain Discharge Percent TSS Reduction (0-100) or leave blank for program to calculate: 0

Select Subgrade Seepage Rate
 Sand - 8 in/hr
 Loamy sand - 2.5 in/hr
 Sandy loam - 1.0 in/hr
 Loam - 0.5 in/hr
 Silt loam - 0.3 in/hr
 Sandy silt loam - 0.2 in/hr
 Clay loam - 0.1 in/hr
 Silty clay loam - 0.05 in/hr
 Sandy clay - 0.05 in/hr
 Silty clay - 0.04 in/hr
 Clay - 0.02 in/hr

Percent of Total Area that is Porous Pavement: 40.0 %

Cleaning Frequencies

Graphic with entered data

16

Porous Pavement Control Device

First Source Area Control Practice
Land Use: Commercial 1
Source Area: Paved Parking 1
Total Porous and Impervious Pavement Area: 1.000 ac.
Porous pavement area (acres): 0.170
Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Surface Pavement Layer Infiltration Rate Data

Initial Infiltration Rate (in/hr): 100.00
Surface Pavement Percent Solids Removal Upon Cleaning (0-100): 50.0

Enter either these three values:
Percent of Infiltration Rate After 3 Years (0-100):
Percent of Infiltration Rate After 5 Years (0-100):
Time Period Until Complete Clogging Occurs (yrs):
Or this value:
Surface Clogging Load (lb/sf): 0.06

Restorative Cleaning Frequency

- Never Cleaned
- Three Times per Year
- Semi-Annually
- Annually
- Every Two Years
- Every Three Years
- Every Four Years
- Every Five Years
- Every Seven Years
- Every Ten Years

Surface Clogging Option 1

1 - Pavement Thickness (in): 3.0
Pavement Porosity (>0 and <1): 0.25
2 - Aggregate Bedding Thickness (in): 9.0
Aggregate Bedding Porosity (>0 and <1): 0.30
3 - Aggregate Base Reservoir Thickness (in): 12.0
Aggregate Base Reservoir Porosity (>0 and <1): 0.30
Porous Pavement Area to Agg Base Area Ratio: 1.00

Outlet/Discharge Options

Perforated Pipe Underdrain Diameter, if used (inches): 3.00
4 - Perforated Pipe Underdrain Outlet Invert Elevation (inches above Datum): 6.0
Number of Perforated Pipe Underdrains (<250): 5
Subgrade Seepage Rate (in/hr) - select below or enter: 0.100
Use Random Number Generation to Account for Uncertainty in Seepage Rate:
Subgrade Seepage Rate COV:
Underdrain Discharge Percent TSS Reduction (0-100) or leave blank for program to calculate: 0

Percent of Total Area that is Porous Pavement: 17.0 %

Porous Pavement Geometry Schematic

Control Practice #: 1 | Land Use #: 1 | Source Area #: 13 | Porous Pavement Device Number 1 | 17

17

Porous Pavement Control Device

First Source Area Control Practice
Land Use: Commercial 1
Source Area: Paved Parking 1
Total Porous and Impervious Pavement Area: 1.000 ac.
Porous pavement area (acres): 0.170
Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Surface Pavement Layer Infiltration Rate Data

Initial Infiltration Rate (in/hr): 100.00
Surface Pavement Percent Solids Removal Upon Cleaning (0-100): 50.0

Enter either these three values:
Percent of Infiltration Rate After 3 Years (0-100):
Percent of Infiltration Rate After 5 Years (0-100):
Time Period Until Complete Clogging Occurs (yrs):
Or this value:
Surface Clogging Load (lb/sf): 0.06

Restorative Cleaning Frequency

- Never Cleaned
- Three Times per Year
- Semi-Annually
- Annually
- Every Two Years
- Every Three Years
- Every Four Years
- Every Five Years
- Every Seven Years
- Every Ten Years

Surface Clogging Option 2

1 - Pavement Thickness (in): 3.0
Pavement Porosity (>0 and <1): 0.25
2 - Aggregate Bedding Thickness (in): 9.0
Aggregate Bedding Porosity (>0 and <1): 0.30
3 - Aggregate Base Reservoir Thickness (in): 12.0
Aggregate Base Reservoir Porosity (>0 and <1): 0.30
Porous Pavement Area to Agg Base Area Ratio: 1.00

Outlet/Discharge Options

Perforated Pipe Underdrain Diameter, if used (inches): 3.00
4 - Perforated Pipe Underdrain Outlet Invert Elevation (inches above Datum): 6.0
Number of Perforated Pipe Underdrains (<250): 5
Subgrade Seepage Rate (in/hr) - select below or enter: 0.100
Use Random Number Generation to Account for Uncertainty in Seepage Rate:
Subgrade Seepage Rate COV:
Underdrain Discharge Percent TSS Reduction (0-100) or leave blank for program to calculate: 0

Percent of Total Area that is Porous Pavement: 17.0 %

Porous Pavement Geometry Schematic

Control Practice #: 1 | Land Use #: 1 | Source Area #: 13 | Porous Pavement Device Number 1 | 18

18

Modeling Notes

- Porous pavement routing is performed using the Modified Puls Storage – Indication Method.
- Time increments are established by the user and vary by event.
- Yield reductions are due to
 - surface pavement filtering
 - subsurface settling
 - runoff volume reduction through infiltration
- The pavement surface can be any material – paver blocks, porous asphalt or porous concrete
- The porous pavement structure is assumed to be flat

19

19

Modeling Notes

- In Northern Climates, groundwater contamination due to Chlorides may be a problem – check with your regulatory agency
- Porous pavement clogging and cleaning time frames may require a multi-year analysis to correctly evaluate performance
- Clogging due to run on increases as the porous pavement area to drainage area ratio decreases

Control Practice No.	Control Practice Name	Total Inflow Volume (cfs)	Total Outflow Volume (cfs)	Total Inflow Load (lb)	Total Outflow Load (lb)	Percent Load Reduction	Flow Weighted Inflow Concentration (mg/l)	Flow Weighted Outflow Concentration (mg/l)	Percent Conc. Reduction	Inflow Median Part. Size (microns)	Effluent Median Part. Size (microns)	% of Design Substrate (Time Int.)	Maximum Value Inflow (lb)	Maximum Value Outflow (lb)	Underdrain Discharge Volume (cfs)	Final Surface Inflow (lb)	Runoff Event/15 Min.	
1	Porous Pavement	75536	37960	4972	613.0	94.06	84.86	130.0	35.07	69.483	7.80	2.33	0.12	66.50	37524.94	37599.71	87.50	21.96

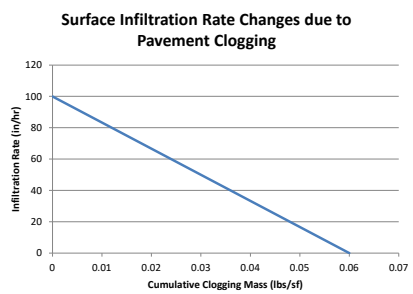
20

20

Surface Seepage Rate Changes due to Surface Clogging

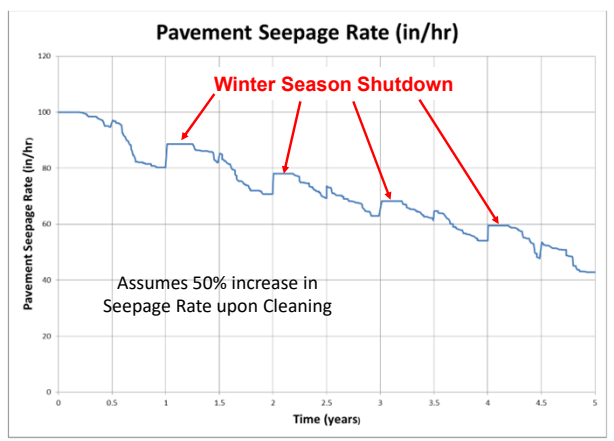
Table 1 - Particulate Treatment in Porous Pavement Devices
Fractional Removal of Stormwater Particulates

Media	0.45 to 3µm	3 to 12µm	12 to 30µm	30 to 60µm	60 to 120µm	120 to 250µm	>250µm
Porous pavement surface (asphalt or concrete)	0.00	0.00	0.00	0.00	0.25	0.50	1.00

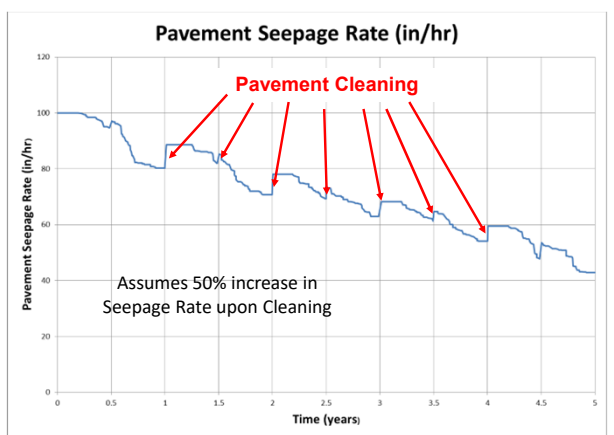


Infiltration rate decreases as sediment clogs the surface

Surface Seepage Rate Changes to the Control Practice



Surface Seepage Rate Changes to the Control Practice



Example Input and Output

WinSLAMM v 10 Data File: [C:\Files\SLAMM\Training]

File Current File Data Pollutants Tools Run Utilities Help

RES INS COP INO LU FFE CS WP BF PP HD OD FS SF LF

Land Use:

Source Area #	Source Area	Area (acres)	Source Area Parameters	First Control Practice	Second Control Practice
Commercial 1					
Roofs					
1	Roofs 1				
2	Roofs 2				
3	Roofs 3				
4	Roofs 4				
5	Roofs 5				
6	Roofs 6				
7	Roofs 7				
8	Roofs 8				
9	Roofs 9				
10	Roofs 10				
11	Roofs 11				
12	Roofs 12				
Parking					
13	Paved Parking 1	1.000	Entered	PP	
14	Paved Parking 2				
15	Paved Parking 3				
16	Paved Parking 4				
17	Paved Parking 5				
18	Paved Parking 6				
19	Unpaved Parking 1				
20	Unpaved Parking 2				
Land Use					
Land Use #	Land Use Type	Land Use Label	Land Use Area (acres)		
1	Commercial	Commercial 1	1.000		
Control Practice					
CP #	Control Practice Type	Control Practice Name or Location			
1	Porous Pavement	SA Device, LU# 1, SA# 13			

Commercial 1

Junction 1

Outlet

Porous Pavement Control Device

First Source Area Control Practice
Land Use: Commercial 1
Source Area: Paved Parking 1
Total Porous and Impervious Pavement Area: 1,000 ac.
Porous pavement area (acres): 0.170
Inflow Hydrograph Peak to Average Flow Ratio: 3.8

Pavement Geometry and Properties

1 - Pavement Thickness (in)	3.0
Pavement Porosity (>0 and <1)	0.25
2 - Aggregate Bedding Thickness (in)	9.0
Aggregate Bedding Porosity (>0 and <1)	0.30
3 - Aggregate Base Reservoir Thickness (in)	12.0
Aggregate Base Reservoir Porosity (>0 and <1)	0.30
Porous Pavement Area to Agg Base Area Ratio	1.00

Outlet/Discharge Options

Perforated Pipe Underdrain Diameter, if used (inches)	3.00
4 - Perforated Pipe Underdrain Outlet Invert Elevation (inches above datum)	6.0
Number of Perforated Pipe Underdrains (<250)	5
Subgrade Seepage Rate (in/hr) - select below or enter	0.100
Use Random Number Generation to Account for Uncertainty in Seepage Rate	<input type="checkbox"/>
Subgrade Seepage Rate COV	
Underdrain Discharge Percent TSS Reduction (0-100) or leave blank for program to calculate	0

Select Subgrade Seepage Rate

- Sand - 8 in/hr
- Loamy sand - 2.5 in/hr
- Sandy loam - 1.0 in/hr
- Loam - 0.5 in/hr
- Silt loam - 0.3 in/hr
- Sandy silt loam - 0.2 in/hr
- Clay loam - 0.1 in/hr
- Silty clay loam - 0.05 in/hr
- Sandy clay - 0.05 in/hr
- Silty clay - 0.04 in/hr
- Clay - 0.02 in/hr

Surface Pavement Layer Infiltration Rate Data

Initial Infiltration Rate (in/hr): 100.00
Surface Pavement Percent Solids Removal Upon Clearing (0-100): 50.0

Enter either these three values:
Percent of Infiltration Rate After 3 Years (0-100):
Percent of Infiltration Rate After 5 Years (0-100):
Time Period Until Complete Clogging Occurs (yrs):

Or this value:
Surface Clogging Load (lb/ft): 0.06

Select Particle Size Distribution File
Select File: Not needed - calculated by program

Restorative Cleaning Frequency

- Never Cleaned
- Three Times per Year
- Semi-Annually
- Annually
- Every Two Years
- Every Three Years
- Every Five Years
- Every Seven Years
- Every Ten Years

Outlet/Discharge Options

Percent of Total Area that is Porous Pavement: 17.0 %

Porous Pavement Geometry Schematic

3.0"
9.0"
24.0"
12.0"
6.0"

Pavement Surface
Porous Pavement Layer
Aggregate Bed Layer
Aggregate Base Layer
Subgrade

Copy Porous Pavement Data Paste Porous Pavement Data

Delete Control Cancel Continue

Control Practice #: 1 Land Use #: 1 Source Area #: 13 Porous Pavement Device Number 1

25

Land Uses
Junctions
Control Practices
Outfall
Output Summary

File Name: C:\Files\SLAMM\Training-Presentations\DNR PP Feb 2015 Webinar\VPPT est.mdb

Outfall Output Summary

	Runoff Volume (cu. ft.)	Percent Runoff Reduction	Runoff Coefficient (Rv)	Particulate Solids Conc. (mg/L)	Particulate Solids Yield (lbs)	Percent Particulate Solids Reduction
Total of All Land Uses without Controls	75538		0.71	130.0	613.0	
Outfall Total with Controls	38001	49.69 %	0.36	39.67	94.11	84.65 %
Current File Output: Annualized Total After Outfall Controls	50994		Years in Model Run: 0.75		126.3	

Pollutant	Concentration - No Controls	Concentration - With Controls	Concentration Units	Pollutant Yield - No Controls	Pollutant Yield - With Controls	Pollutant Yield Units	Percent Yield Reduction
Particulate Solids	130.0	39.67	mg/L	613.0	94.11	lbs	84.65 %
Total Phosphorus	0.2150	0.08646	mg/L	1.014	0.2051	lbs	79.77 %

Total Control Practice Costs

Capital Cost	\$ 14730
Land Cost	\$ 0
Annual Maintenance Cost	\$ 49
Present Value of All Costs	\$ 15463
Annualized Value of All Costs	\$ 1039

Receiving Water Impacts Due To Stormwater Runoff (CWP Impervious Cover Model)

	Calculated Rv	Approximate Urban Stream Classification
Without Controls	0.71	Poor
With Controls	0.36	Poor

Total Area Modeled (ac): 1.000

Print Output Summary to Text File Print Output Summary to csv File Perform Outfall Flow Duration Curve Calculations

26

Land Uses
Junctions
Control Practices
Outfall
Output Summary

File Name: C:\Files\SLAMM\Training-Presentations\DNR PP Feb 2015 Webinar\VPPT est.mdb

Outfall Output Summary

	Runoff Volume (cu. ft.)	Percent Runoff Reduction	Runoff Coefficient (Rv)	Particulate Solids Conc. (mg/L)	Particulate Solids Yield (lbs)	Percent Particulate Solids Reduction
Total of All Land Uses without Controls	75538		0.71	130.0	613.0	
Outfall Total with Controls	38001	49.69 %	0.36	39.67	94.11	84.65 %
Current File Output: Annualized Total After Outfall Controls	50994		Years in Model Run: 0.75		126.3	

Pollutant	Concentration - No Controls	Concentration - With Controls	Concentration Units	Pollutant Yield - No Controls	Pollutant Yield - With Controls	Pollutant Yield Units	Percent Yield Reduction
Particulate Solids	130.0	39.67	mg/L	613.0	94.11	lbs	84.65 %
Total Phosphorus	0.2150	0.08646	mg/L	1.014	0.2051	lbs	79.77 %

Total Control Practice Costs

Capital Cost	\$ 14730
Land Cost	\$ 0
Annual Maintenance Cost	\$ 49
Present Value of All Costs	\$ 15463
Annualized Value of All Costs	\$ 1039

Receiving Water Impacts Due To Stormwater Runoff (CWP Impervious Cover Model)

	Calculated Rv	Approximate Urban Stream Classification
Without Controls	0.71	Poor
With Controls	0.36	Poor

Total Area Modeled (ac): 1.000

Print Output Summary to Text File Print Output Summary to csv File Perform Outfall Flow Duration Curve Calculations

27

Land Uses
Junctions
Control Practices
Outfall
Output Summary

Data File: C:\Files\SLAMM\Training-Presentations\DNR PP Feb 2015 Webinar\VPPT est.mdb
Run File: WinReg - Madison WI 1981.RAN
Date: 01-26-15 Time: 12:50:06 PM
Site Description:

Col. #:	1	2	3	4	5	6	7	8	9	10
Control Practice No.	Control Practice Type	Control Practice Name or Location	Total Inflow Volume (cf)	Total Outflow Volume (cf)	Percent Volume Reduction	Total Infiltrant Load (lbs)	Total Effluent Load (lbs)	Percent Load Reduction	Flow Weight Infiltrant Conc (m	
1	Porous Pavement	SA Device, LU# 1 SA# 13	75538	38001	49.69	613.0	94.12	84.65	1	

28

Control Practice Summary Table

Data File: C:\Files\SLAMM\Training-Presentations\DNR PP Feb 2015 Webinar\PPTest.mdb
 Rain File: WisReg - Madison WI 1981.RAN
 Date: 01-25-15 Time: 12:38:41 PM
 Site Description:

Col. #	2	3	4	5	6	7	8	9
Control Practice No.	Control Practice Type	Control Practice Name or Location	Total Inflow Volume (cf)	Total Outflow Volume (cf)	Percent Volume Reduction	Total Influent Load (lbs)	Total Effluent Load (lbs)	Percent Load Reduction
1	Porous Pavement	SA Device, LU# 1, SA# 13	75538	37980	49.72	613.0	94.06	84.66

10	11	12	13	14	26	28	29	30	36	61
Flow Weighted Influent Conc (mg/L)	Flow Weighted Effluent Conc (mg/L)	Percent Reduction	Influent Median Part. Size (microns)	Effluent Median Part. Size (microns)	% of Clogging Factor	Maximum Subsurface Pounding Time (hrs)	Volume Infiltrated (cf)	Underdrain Discharge Vol. (cf)	Final Surface Infiltration Rate (in/hr)	Runoff Producing Events/ Tot. Rains
130.0	39.67	69.483	7.80	2.33	0.12	68.50	37524.94	37979.71	87.93	21/86

29

29

Control Practice Detail Tables

Runoff Volume

Data File: C:\Files\SLAMM\Training-Presentations\DNR PP Feb 2015 Webinar\PPTest.mdb
 Rain File: WisReg - Madison WI 1981.RAN
 Date: 01-25-15 Time: 12:38:40 PM
 Site Description:

Control Practice Type ==>	CP# 1 - Porous Pavement					
Control Practice Name/Location ==>	SA Device, LU# 1, SA# 13					
Rain Number	Start Date	Rain Total (in)	Influent Runoff Vol (cf)	Effluent Runoff Vol (cf)	Runoff Vol. Percent Reduction	
36	05/13/81	0.01	3.227	0	100.00	
37	05/23/81	0.02	12.91	0	100.00	
38	05/24/81	0.10	169.5	0	100.00	
39	05/29/81	0.34	763.0	0	100.00	
40	06/02/81	0.01	3.227	0	100.00	
41	06/03/81	0.01	3.227	0	100.00	
42	06/08/81	0.01	3.227	0	100.00	
43	06/08/81	0.33	735.6	0	100.00	
44	06/09/81	0.07	106.7	0	100.00	
45	06/12/81	0.43	1013	0	100.00	
46	06/15/81	2.59	861.0	7311	15.09	
47	06/20/81	0.34	763.0	0	100.00	
48	06/21/81	0.32	708.6	0	100.00	
49	06/23/81	0.51	1240	0	100.00	
50	06/25/81	0.13	236.2	0	100.00	
51	06/29/81	0.24	503.1	0	100.00	
52	07/04/81	0.05	67.94	0	100.00	
53	07/11/81	0.50	1211	56.08	95.37	
54	07/12/81	0.14	268.9	21.34	91.53	
55	07/12/81	0.86	2325	2075	10.74	
56	07/13/81	1.32	3966	3778	4.72	
57	07/14/81	0.12	214.2	0	100.00	
--	--	--	--	--	--	

Part Solids Yield (lbs)

Data File: C:\Files\SLAMM\Training-Presentations\DNR PP Feb 2015 Webinar\PPTest.mdb
 Rain File: WisReg - Madison WI 1981.RAN
 Date: 01-25-15 Time: 12:38:41 PM
 Site Description:

Control Practice Type ==>	CP# 1 - Porous Pavement					
Control Practice Name/Location ==>	SA Device, LU# 1, SA# 13					
Rain Number	Start Date	Rain Total (in)	Influent Part Sol. Yield (lbs)	Effluent Part Sol. Yield (lbs)	Part Yield Percent Reduction	
36	05/13/81	0.01	0.02619	0	100.00	
37	05/23/81	0.02	0.1048	0	100.00	
38	05/24/81	0.10	1.376	0	100.00	
39	05/29/81	0.34	6.192	0	100.00	
40	06/02/81	0.01	0.02619	0	100.00	
41	06/03/81	0.01	0.02619	0	100.00	
42	06/08/81	0.01	0.02619	0	100.00	
43	06/08/81	0.33	5.970	0	100.00	
44	06/09/81	0.07	0.8662	0	100.00	
45	06/12/81	0.43	8.224	0	100.00	
46	06/15/81	2.59	69.88	22.59	67.67	
47	06/20/81	0.34	6.192	0	100.00	
48	06/21/81	0.32	5.751	0	100.00	
49	06/23/81	0.51	10.06	0	100.00	
50	06/25/81	0.13	1.917	0	100.00	
51	06/29/81	0.24	4.063	0	100.00	
52	07/04/81	0.05	0.5513	0	100.00	
53	07/11/81	0.50	9.825	0.02718	99.72	
54	07/12/81	0.14	2.101	0.004246	99.80	
55	07/12/81	0.86	18.87	7.473	60.40	
56	07/13/81	1.32	32.18	12.83	60.13	
57	07/14/81	0.12	1.738	0	100.00	

30

Additional Output

Available through:
Tools/
Default Model Options

- > Water Balance File
- > Mass Balance File
- > Stage Outflow File
- > Surface Seepage Rate File
- > Detailed Output File
- > Stochastic Seepage Rate Detail File

PorPav Source Area Number	Rain Number	Rain Depth (in)	Time (Julian Date)	Maximum PorPav Stage (ft)	Minimum PorPav Stage (ft)	Total Source Area Runoff Before Porous Pavement (ac-ft)	Non-Porous Pavement Area Runoff Volume (ac-ft)	Event Inflow Volume onto Porous Pavement (ac-ft)	Event Bypass Volume Due to Surface Clogging (ac-ft)	Event Overflow Volume (ac-ft)	Event Infiltration Volume (ac-ft)	Event Orifice Outflow Volume (ac-ft)	Event Total Outflow Volume (ac-ft)	Event Flow Balance (ac-ft)	Volume Reduction Fraction	Solids Reduction Fraction
46	1	0.46	0	0.01	0	0.01	0	0.01	0	0	0.01	0	0.01	0	1	0
46	2	0.58	5	0	0	0.012	0	0.012	0	0	0.012	0	0.012	0	1	0
46	3	0.25	9	0	0	0.005	0	0.005	0	0	0.005	0	0.005	0	1	0
46	4	0.03	11	0	0	0.001	0	0.001	0	0	0.001	0	0.001	0	1	0
46	5	0.39	11	0	0	0.008	0	0.008	0	0	0.008	0	0.008	0	1	0
46	7	0.05	18	0	0	0.001	0	0.001	0	0	0.001	0	0.001	0	1	0
46	8	0.03	22	0	0	0.001	0	0.001	0	0	0.001	0	0.001	0	1	0
46	9	2.33	23	0.01	0	0.049	0	0.049	0	0	0.049	0	0.049	0	1	0
46	12	0.51	34	0	0	0.011	0	0.011	0	0	0.011	0	0.011	0	1	0
46	15	0.67	47	0.01	0	0.014	0	0.014	0	0	0.014	0	0.014	0	1	0
46	16	0.61	50	0.01	0	0.013	0	0.013	0	0	0.013	0	0.013	0	1	0
46	18	0.85	63	0	0	0.018	0	0.018	0	0	0.018	0	0.018	0	1	0
46	20	1.02	66	0.01	0	0.021	0	0.021	0	0	0.021	0	0.021	0	1	0
46	22	1.48	70	0.01	0	0.031	0	0.031	0	0	0.031	0	0.031	0	1	0

31

31

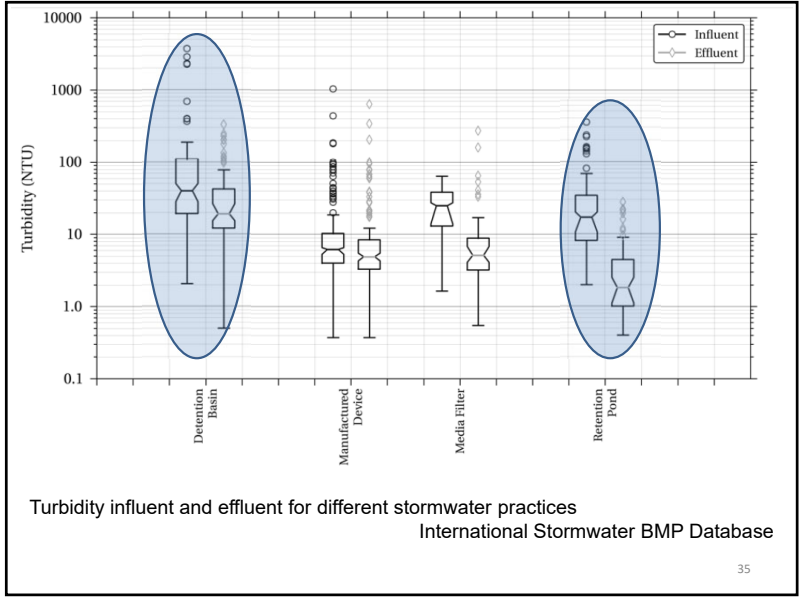
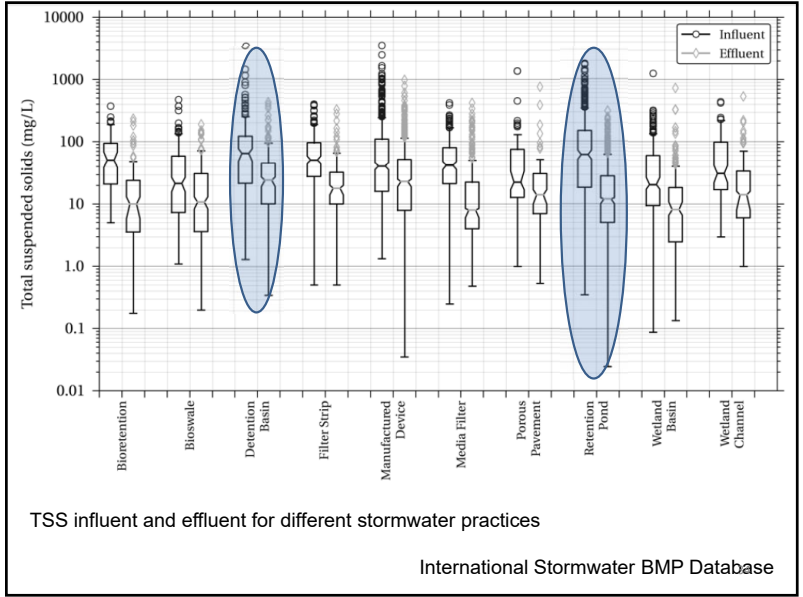
Modeling Wet Detention Ponds in WinSLAMM ver. 10.3

32

32

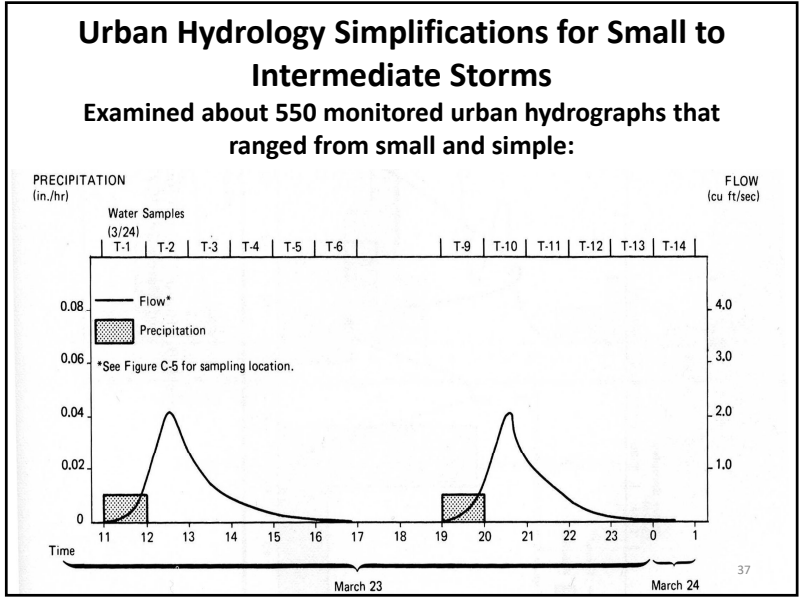
We will cover . .

- Pond Performance
- Entering Wet Detention Pond Data into the

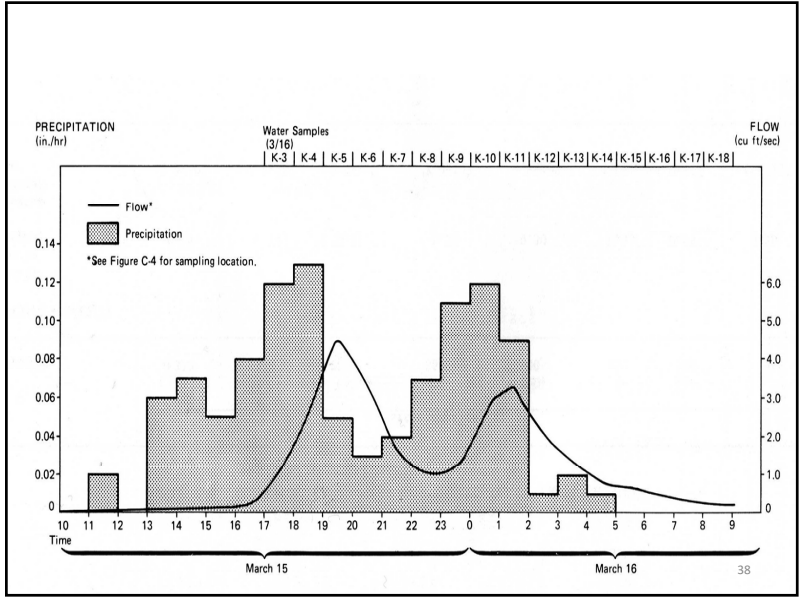


Why is there such a large difference in performance between a dry and a wet pond?

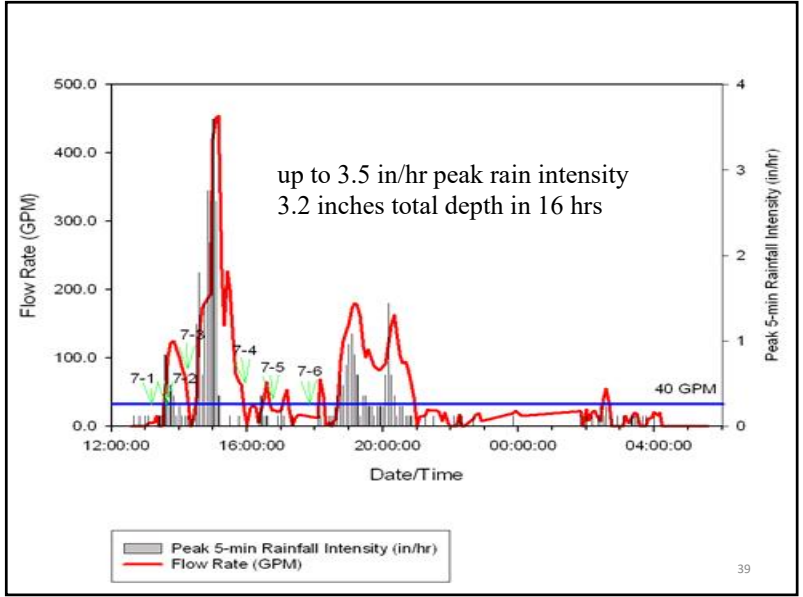
- Usually due to scour
 - Need at least 3 ft for wet ponds to protect previously captured silt
 - Grass filters look like dry ponds (and the grass filters can work well, but require lengthy sheetflows with level spreaders, low slopes, good grass stands, no pilot channels, etc.)
 - Terminology issues (in many areas, dry ponds are actually percolation ponds or infiltration ponds with no surface discharges). HIGs



37



38



39

Observed Urban Hydrographs

Evaluated about 550 different urban area hydrographs from 8 watersheds (1, 1a, 2, and 3 rain distributions and B soils to pavement)

Location	Land use	area (acres)	directly connected impervious	# of events monitored
Bellevue, WA				
Surrey Downs	Resid, med. den.	95	17 %	196
Lake Hills	Resid, med. den.	102	17	201
San Jose, CA				
Keyes	Resid, med. den.	92	30	6
Tropicana	Resid, med. den.	195	25	8
Toronto, Ontario				
Thistledowns	Resid, med. den.	96	21	35
Emery	Industrial	381	42	60
Tuscaloosa, AL				
City Hall	Institutional/com	0.9	100	31
BamaBelle	Commercial	0.9	68	17

40

40

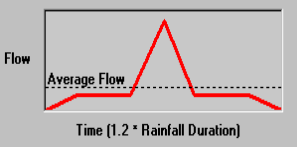
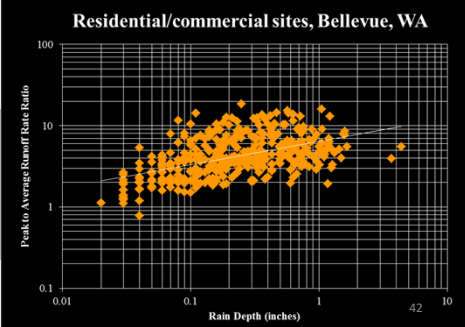
Observed Runoff Characteristics

	Monitored rains (inches, range)	Observed Rv (average)	Observed CN (range)	peak/avg flow ratio (average)
Bellevue, WA				
Surrey Downs	0.03 - 4.38	0.18	64 - 100	4.4
Lake Hills	0.02 - 3.69	0.21	73 - 100	5.4
San Jose, CA				
Keyes	0.01 - 1.06	0.10	88 - 100	3.2
Tropicana	0.01 - 1.08	0.59	95 - 100	3.8
Toronto, Ontario				
Thistledowns	0.03 - 1.01	0.17	84 - 99	4.0
Emery	0.03 - 1.0	0.23	87 - 99	3.1
Tuscaloosa, AL				
City Hall	0.02 - 3.2	0.60	95 - 99	4.2
BamaBelle	0.1 - 1.9	0.80	94 - 100	5.5

41

Complex Hydrograph Creation by WinSLAMM




- Flow rate calculated using Complex Triangular Hydrograph, based on statistical analysis of hundreds of monitored events throughout North America and for different land uses
 - Runoff Volume calculated from WinSLAMM
 - Runoff Duration = 1.2 times rainfall duration
 - Peak to average flow rate ratio user defined, but usually about 3.8

42

Modeling Notes

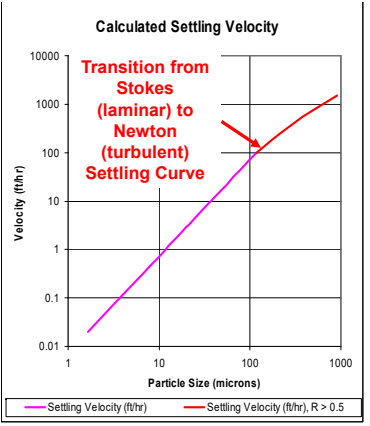
- WinSLAMM assumes a 3.0 ft scour depth for complete settling; reduces treatment effectiveness for shallower depths.
- Pond routing is performed using the Modified Puls – Storage Indication Method.
- Time increments are established by the user; default = 6 minutes.

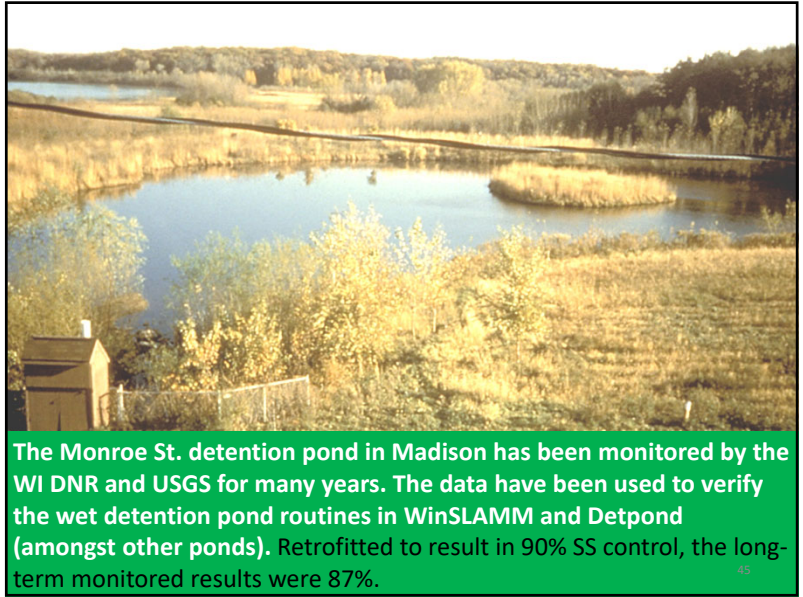
43

Particulate Settling

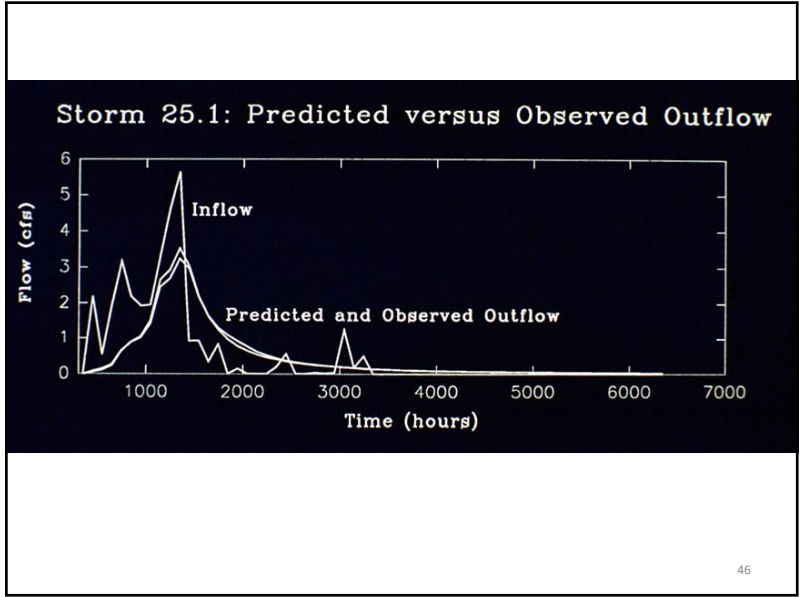
- Ideal settling is modeled
 - Using Stokes Law (laminar flow) for smaller particles
 - Settling velocity as a function of Reynolds number and particle size for larger particles under turbulent flow conditions
 - Water temperature (can vary monthly) and particle density also affect settling rates and can be changed in WinSLAMM



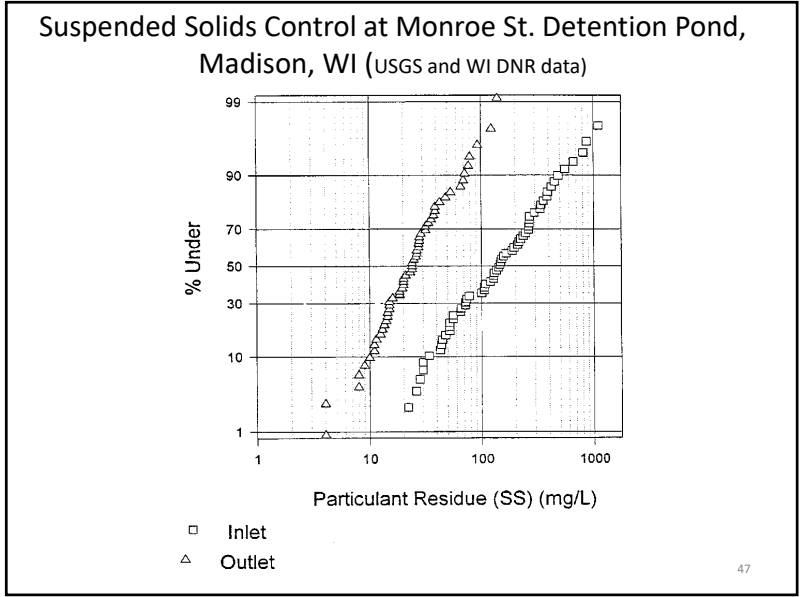
44



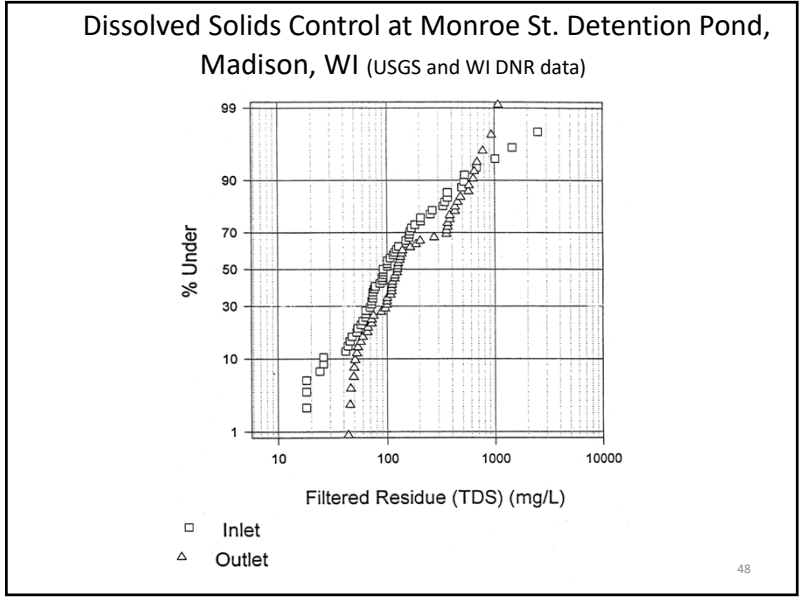
45



46

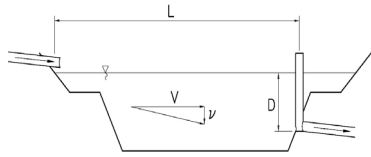


47



48

Particulate Settling



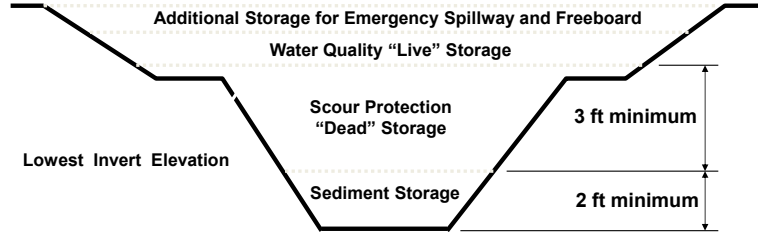
- **Ideal Settling: Particle path is vector sum of particle velocity through pond and settling (upflow) velocity**

$$\frac{V}{v} = \frac{L}{D} \rightarrow v = \frac{Q_{out}}{A}$$

As shown on pages 23-25 of detention pond design.pdf and outlined on the following ppts

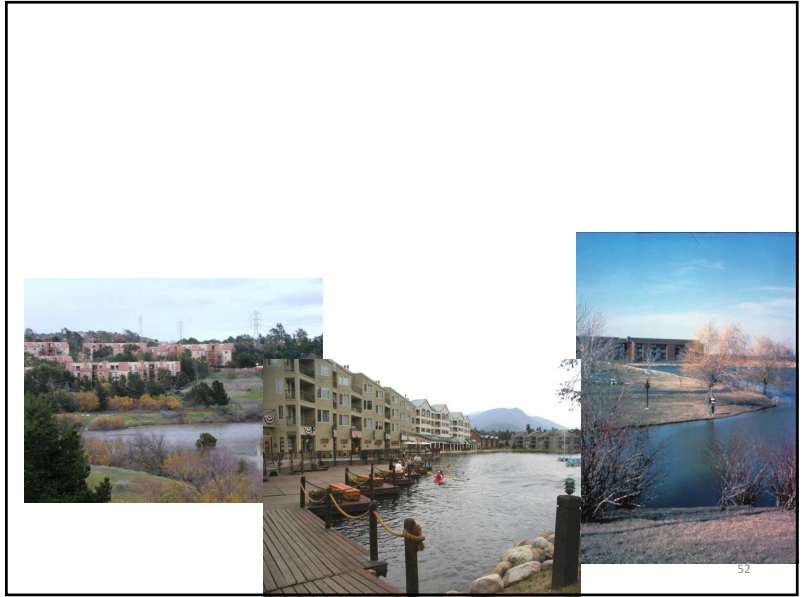
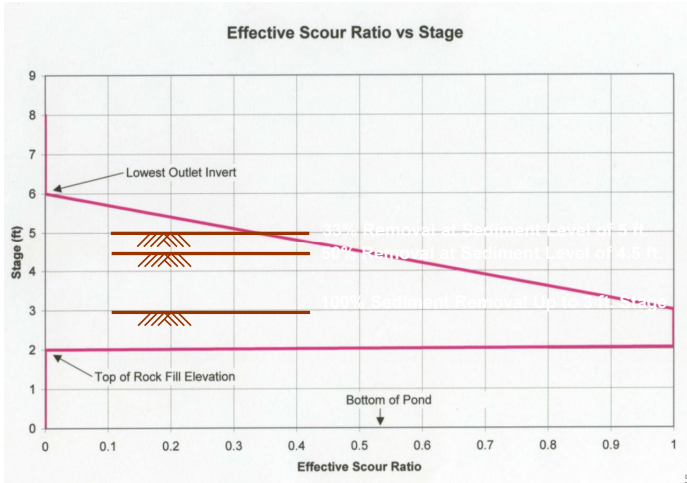
- > L = Pond Length
- > D = Outlet Depth
- > V = Water Velocity through Pond
- > v = Settling Velocity
- > Q_{out} = Outflow from Pond
- > A = Pond Surface Area

Conceptual Issues – Pond Geometry and Scour



The “dead” storage is needed to prevent scour of previously deposited material and should be at least 3 ft. deep over the sediment. Sediment storage volume is also needed and can be estimated using the program, or should be at least 2 ft. deep.

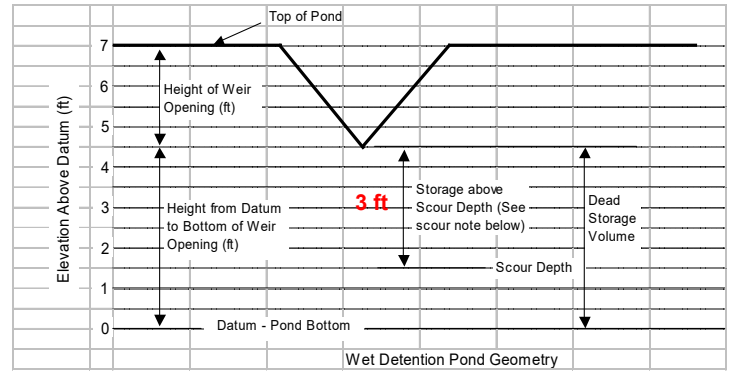
Scour in WinSLAMM



Wet Detention Pond Performance Calculation Data Requirements

- Surface area of pond
- Water quality volume (live storage above lowest pond water surface elevation, usually the pond volume between the water quality outlet and the emergency spillway)
- Depth of water over the sediment to prevent scour
- Stage-discharge relationship for all outlets
- Particle size distribution of inflowing
- Hydrograph of influent flows

Pond Geometry



Pond Datum is always zero ft.

Wet Detention Pond Data Entry

Flow, Initial Stage and Particle Size Information

Particle Size Distribution File not accessible if Flows and Particle Sizes transferred through the drainage system

Pond Outlet Information

Wet Detention Control Device

Pond Number 1
Drainage System Control Practice

Select Particle Size Distribution File
C:\Program Files (x86)\WinSLAMM
v10\NURP.CPZ

Initial Stage Elevation (ft): 3.00
Peak to Average Flow Ratio: 3.80

Broad Crested Weir

WEIR CREST WIDTH

Flow

h_3

Stage (ft)	Area (acres)	Cumulative Volume (ac-ft)
0	0.00	0.000
1	1.00	0.500
2	2.00	1.000
3	3.00	1.500
4	4.00	2.000
5	5.00	2.500
6	6.00	3.000

Remove	Sharp Crested Weir	Remove	Add
Weir Length (ft)	20.00		
Height from datum to bottom of weir opening (ft)	5.00		

Remove	V-Notch Weir	Remove	Add
Weir Angle (<180 degrees)	60		
Height from datum to bottom of weir opening (ft)	3.50		
Number of V-Notch weirs	3		

Remove	Orifice Set 1	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.00		
Number of orifices in set	2		

Remove	Orifice Set 2	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.50		
Number of orifices in set	2		

Remove	Orifice Set 3	Remove	Add
Orifice Diameter (ft)	0.00		
Invert elevation above datum (ft)	0.00		
Number of orifices in set	0		

Remove	Stone Weeper	Remove	Add
Width at bottom of weeper (ft)	4.00		
Weeper side slope (L:H:TV)	2.00		
Upstream side slope (L:H:TV)	3.00		
Downstream side slope (L:H:TV)	3.00		
Horizontal flow path length at top of weeper (ft)	2.00		
Average rock diameter (ft)	0.30		
Distance from bottom to top of weeper (ft)	2.00		
Height from datum to bottom of weeper (ft)	4.00		

Remove	Broad Crested Weir	Remove	Add
Weir crest length (ft)	10.00		
Weir crest width (ft)	3.00		
Height of weir opening (ft)	0.50		
Height from datum to bottom of weir opening (ft)	5.50		

Remove	Seepage Basin	Remove	Add
Infiltration rate (in/hr)	0.00		
Width of device (ft)	0.00		
Length of device (ft)	0.00		
Invert elevation of seepage basin inlet above datum (ft)	0.00		

Remove	Vertical Stand Pipe	Remove	Add
Pipe diameter (ft)	2.00		
Height above datum (ft)	5.00		

Control Practice #: 1 CP Element #: 1 57

57

Wet Detention Control Device

Pond Number 1
Drainage System Control Practice

Select Particle Size Distribution File
C:\Program Files (x86)\WinSLAMM
v10\NURP.CPZ

Initial Stage Elevation (ft): 3.00
Peak to Average Flow Ratio: 3.80

Orifice

ORIFICE DIAMETER

Flow

h_2

POND DATUM

Stage (ft)	Area (acres)	Cumulative Volume (ac-ft)
0	0.00	0.000
1	1.00	0.500
2	2.00	1.000
3	3.00	1.500
4	4.00	2.000
5	5.00	2.500
6	6.00	3.000

Remove	Sharp Crested Weir	Remove	Add
Weir Length (ft)	20.00		
Height from datum to bottom of weir opening (ft)	5.00		

Remove	V-Notch Weir	Remove	Add
Weir Angle (<180 degrees)	60		
Height from datum to bottom of weir opening (ft)	3.50		
Number of V-Notch weirs	3		

Remove	Orifice Set 1	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.00		
Number of orifices in set	2		

Remove	Orifice Set 2	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.50		
Number of orifices in set	2		

Remove	Orifice Set 3	Remove	Add
Orifice Diameter (ft)	0.00		
Invert elevation above datum (ft)	0.00		
Number of orifices in set	0		

Remove	Stone Weeper	Remove	Add
Width at bottom of weeper (ft)	4.00		
Weeper side slope (L:H:TV)	2.00		
Upstream side slope (L:H:TV)	3.00		
Downstream side slope (L:H:TV)	3.00		
Horizontal flow path length at top of weeper (ft)	2.00		
Average rock diameter (ft)	0.30		
Distance from bottom to top of weeper (ft)	2.00		
Height from datum to bottom of weeper (ft)	4.00		

Remove	Broad Crested Weir	Remove	Add
Weir crest length (ft)	10.00		
Weir crest width (ft)	3.00		
Height of weir opening (ft)	0.50		
Height from datum to bottom of weir opening (ft)	5.50		

Remove	Seepage Basin	Remove	Add
Infiltration rate (in/hr)	0.00		
Width of device (ft)	0.00		
Length of device (ft)	0.00		
Invert elevation of seepage basin inlet above datum (ft)	0.00		

Remove	Vertical Stand Pipe	Remove	Add
Pipe diameter (ft)	2.00		
Height above datum (ft)	5.00		

Control Practice #: 1 CP Element #: 1 58

58

Wet Detention Control Device

Pond Number 1
Drainage System Control Practice

Select Particle Size Distribution File
C:\Program Files (x86)\WinSLAMM
v10\NURP.CPZ

Initial Stage Elevation (ft): 3.00
Peak to Average Flow Ratio: 3.80

Stone Weeper

Top of Pond

Flow

h

Stone Weeper Cross Section

Stage (ft)	Area (acres)	Cumulative Volume (ac-ft)
0	0.00	0.000
1	1.00	0.500
2	2.00	1.000
3	3.00	1.500
4	4.00	2.000
5	5.00	2.500
6	6.00	3.000

Remove	Sharp Crested Weir	Remove	Add
Weir Length (ft)	20.00		
Height from datum to bottom of weir opening (ft)	5.00		

Remove	V-Notch Weir	Remove	Add
Weir Angle (<180 degrees)	60		
Height from datum to bottom of weir opening (ft)	3.50		
Number of V-Notch weirs	3		

Remove	Orifice Set 1	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.00		
Number of orifices in set	2		

Remove	Orifice Set 2	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.50		
Number of orifices in set	2		

Remove	Orifice Set 3	Remove	Add
Orifice Diameter (ft)	0.00		
Invert elevation above datum (ft)	0.00		
Number of orifices in set	0		

Remove	Stone Weeper	Remove	Add
Width at bottom of weeper (ft)	4.00		
Weeper side slope (L:H:TV)	2.00		
Upstream side slope (L:H:TV)	3.00		
Downstream side slope (L:H:TV)	3.00		
Horizontal flow path length at top of weeper (ft)	2.00		
Average rock diameter (ft)	0.30		
Distance from bottom to top of weeper (ft)	2.00		
Height from datum to bottom of weeper (ft)	4.00		

Remove	Broad Crested Weir	Remove	Add
Weir crest length (ft)	10.00		
Weir crest width (ft)	3.00		
Height of weir opening (ft)	0.50		
Height from datum to bottom of weir opening (ft)	5.50		

Remove	Seepage Basin	Remove	Add
Infiltration rate (in/hr)	0.00		
Width of device (ft)	0.00		
Length of device (ft)	0.00		
Invert elevation of seepage basin inlet above datum (ft)	0.00		

Remove	Vertical Stand Pipe	Remove	Add
Pipe diameter (ft)	2.00		
Height above datum (ft)	5.00		

Control Practice #: 1 CP Element #: 1 59

59

Wet Detention Control Device

Pond Number 1
Drainage System Control Practice

Select Particle Size Distribution File
C:\Program Files (x86)\WinSLAMM
v10\NURP.CPZ

Initial Stage Elevation (ft): 3.00
Peak to Average Flow Ratio: 3.80

V-Notch Weir

Flow

h

Stage (ft)	Area (acres)	Cumulative Volume (ac-ft)
0	0.00	0.000
1	1.00	0.500
2	2.00	1.000
3	3.00	1.500
4	4.00	2.000
5	5.00	2.500
6	6.00	3.000

Remove	Sharp Crested Weir	Remove	Add
Weir Length (ft)	20.00		
Height from datum to bottom of weir opening (ft)	5.00		

Remove	V-Notch Weir	Remove	Add
Weir Angle (<180 degrees)	60		
Height from datum to bottom of weir opening (ft)	3.50		
Number of V-Notch weirs	3		

Remove	Orifice Set 1	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.00		
Number of orifices in set	2		

Remove	Orifice Set 2	Remove	Add
Orifice Diameter (ft)	0.25		
Invert elevation above datum (ft)	3.50		
Number of orifices in set	2		

Remove	Orifice Set 3	Remove	Add
Orifice Diameter (ft)	0.00		
Invert elevation above datum (ft)	0.00		
Number of orifices in set	0		

Remove	Stone Weeper	Remove	Add
Width at bottom of weeper (ft)	4.00		
Weeper side slope (L:H:TV)	2.00		
Upstream side slope (L:H:TV)	3.00		
Downstream side slope (L:H:TV)	3.00		
Horizontal flow path length at top of weeper (ft)	2.00		
Average rock diameter (ft)	0.30		
Distance from bottom to top of weeper (ft)	2.00		
Height from datum to bottom of weeper (ft)	4.00		

Remove	Broad Crested Weir	Remove	Add
Weir crest length (ft)	10.00		
Weir crest width (ft)	3.00		
Height of weir opening (ft)	0.50		
Height from datum to bottom of weir opening (ft)	5.50		

Remove	Seepage Basin	Remove	Add
Infiltration rate (in/hr)	0.00		
Width of device (ft)	0.00		
Length of device (ft)	0.00		
Invert elevation of seepage basin inlet above datum (ft)	0.00		

Remove	Vertical Stand Pipe	Remove	Add
Pipe diameter (ft)	2.00		
Height above datum (ft)	5.00		

Control Practice #: 1 CP Element #: 1 60

60

Wet Detention Control Device

Pond Number 1

Drainage System Control Practice: Sharp Crested Weir

Remove | Sharp Crested Weir

Month	Evaporation (in/day)	Water Withdrawal Rate (ac-ft/day)
Jan	0.00	0.00
Feb	0.00	0.00
Mar	0.00	0.00
Apr	0.20	0.00
May	0.10	0.00
Jun	0.40	0.00
Jul	0.50	0.00
Aug	0.50	0.00
Sep	0.30	0.00
Oct	0.10	0.00
Nov	0.00	0.00
Dec	0.00	0.00

Evaporation and Water Withdraw Rate

Natural Seepage and Other Outflow

Stage (ft)	Natural Seepage Rate (in/day)	Other Outflow (in/day)
1.00	0.00	0.00
2.00	0.00	0.00
3.00	0.00	0.500
4.00	0.00	1.000
5.00	0.00	1.250
6.00	0.00	1.400

Seepage Basin

Vertical Stand Pipe

Remove | Broad Crested Weir

Remove	Add
Weir crest length (ft)	10.00
Weir crest width (ft)	3.00
Height of weir opening (ft)	0.50
Height from datum to bottom of weir opening (ft)	5.50

Remove | Seepage Basin

Remove	Add
Infiltration rate (in/hr)	0.00
Width of device (ft)	0.00
Length of device (ft)	0.00
Invert elevation of seepage basin inlet above datum (ft)	0.00

Vertical Stand Pipe

Remove	Add
Pipe diameter (ft)	2.00
Height above datum (ft)	5.00

Flow

Average Flow

Time (1.2 * Rainfall Duration)

Save this Pond as a WinDETPOD File

Control Practice # : 1 | CP Element # : 1

61

Outfall Output Summary

Total of All Land Uses without Controls	Runoff Volume (cu ft)	Percent Runoff Reduction	Runoff Coefficient (Rv)	Particulate Solids Conc. (mg/L)	Particulate Solids Yield (lbs)	Percent Particulate Solids Reduction
	3.782E+06	0.00%	0.33	145.6	34366	
Total with Controls	3.782E+06	0.00%	0.33	48.19	11377	66.90%

Current File Output - Annualized Total After Outfall Controls

Current File Output - Annualized Total After Outfall Controls	Years in Model Run	Particulate Solids Yield (lbs)	Percent Yield Reduction
3.910E+06	0.97	11763	

Polutant

Polutant	Concentration - No Controls	Concentration - With Controls	Polutant Yield - No Controls	Polutant Yield - With Controls	Percent Yield Reduction
Particulate Phosphorus	0.2974	0.09376 mg/L	70.21	23.95 lbs	66.46%
Filtrable Phosphorus	0.08451	0.0451 mg/L	19.95	19.95 lbs	0%
Total Phosphorus	0.3819	0.1843 mg/L	90.17	43.50 lbs	51.75%

A pond has overflowed during a model run. Review outfall runoff volume event-by-event output to determine which pond it is.

Receiving Water Impacts Due To Stormwater Runoff (CWP Impervious Cover Model)

Without Controls	Calculated Rv	Approximate Urban Stream Classification
0.33		Poor
With Controls	Calculated Rv	Approximate Urban Stream Classification
0.33		Poor

62

A pond has overflowed during a model run. Review outfall runoff volume event-by-event output to determine which pond it is.

63

Outfall Runoff Volume

Pond Outlet Structure Failure (overtopping)

Maximum Flushing Ratio

Maximum Peak Reduction Factor

Maximum Stage

Summary Table

Control Practice No.	Control Practice Type	Notes	Maximum Flushing Ratio	Maximum Peak Reduction Factor	Maximum Stage (ft)	Hydraulic Volume Out (cf)	Minimum Volume (cf)	Runoff Producing Events/ Tot Rains
1	Wet Detention Pond	Pond Overflow Rain #: 507 531 539 549	5.2	1.00	11.96	3782089	68903	76/76

64

Additional Output

- Stage Outflow
- Stone Sweeper Flow
- Detailed Output by Time Step
- Pond Water Balance

Detention Area	Pond Number	Rain Depth (in)	Water Balance Time (Julian Date)	Performan Summary, by Pond Stage (ft)	Minimum Pond Stage (ft)	Event Inflow Volume (ac-ft)	Event Hyd Outflow (ac-ft)	Event Infi Outflow (ac-ft)	Event Evap Outflow (ac-ft)	Event Wtr Wdrt Outflow (ac-ft)	Event Total Outflow (ac-ft)	Event Flow Balance (ac-ft)	Total Outflow (ac-ft)	Cum Flow Balance (ac-ft)
162	1	0.46	0	3.67	3	0.63	0.6	0	0	0	0.6	0.03	0.6	0.03
162	2	0.58	5	3.57	3.06	0.843	0.834	0	0	0	0.834	0.009	1.434	0.039
162	3	0.25	9	3.38	3.08	0.277	0.249	0	0	0	0.249	0.029	1.683	0.068
162	4	0.03	11	3.14	3.12	0.009	0.013	0	0	0	0.013	-0.004	1.696	0.064
162	5	0.39	11	3.55	3.08	0.476	0.501	0	0	0	0.501	-0.025	2.198	0.039
162	6	0.01	14	3.08	3.03	0	0.026	0	0	0	0.026	-0.026	2.224	0.013
162	7	0.05	18	3.08	3.03	0.029	0.029	0	0	0	0.029	0	2.253	0.013
162	8	0.03	22	3.04	3.03	0.009	0.006	0	0	0	0.006	0.003	2.259	0.016
162	9	2.33	23	4.35	3.03	5.329	5.326	0	0	0	5.326	0.004	7.585	0.02
162	10	0.01	30	3.04	3.03	0	0.002	0	0	0	0.002	-0.002	7.587	0.017
162	11	0.01	30	3.03	3.01	0	0.012	0	0	0	0.012	-0.012	7.599	0.005
162	12	0.51	34	3.57	3.01	0.71	0.694	0	0	0	0.694	0.015	8.293	0.021
162	13	0.01	40	3.04	3.01	0	0.018	0	0	0	0.018	-0.018	8.312	0.003
162	14	0.01	46	3.01	3.01	0	0	0	0	0	0	0	8.312	0.003
162	15	0.67	47	3.94	3.01	1.026	0.991	0	0	0	0.991	0.035	9.303	0.038
162	16	0.61	50	3.86	3.08	0.896	0.8	0	0	0	0.8	0.096	10.103	0.134
162	17	0.01	51	3.25	3	0	0.133	0	0	0	0.133	-0.133	10.236	0.001
162	18	0.85	63	3.78	3	1.28	1.22	0	0	0	1.22	0.06	11.455	0.062
162	19	0.01	66	3.12	3.1	0	0.013	0	0	0	0.013	-0.013	11.468	0.049
162	20	1.02	66	3.94	3.1	1.56	1.402	0	0	0	1.402	0.158	12.87	0.207
162	21	0.01	67	3.38	3.08	0	0.166	0	0	0	0.166	-0.166	13.036	0.041
162	22	1.48	70	4.23	3.08	2.524	2.442	0	0	0	2.442	0.082	15.479	0.123
162	23	0.01	71	3.24	3.15	0	0.046	0	0	0	0.046	-0.046	15.524	0.078
162	24	0.01	72	3.15	3.1	0	0.028	0	0	0	0.028	-0.028	15.552	0.05
162	25	3.64	73	4.82	3.06	11.492	11.511	0	0	0	11.511	-0.019	27.063	0.031
162	26	0.04	78	3.1	3.06	0.022	0.008	0	0	0	0.008	0.014	27.071	0.045